

**Bridge Number 2934, Route 4 over Sandy River in Madrid**  
**Final Design Hydrology and Hydraulics Report**



**Maine DOT WIN 22615.00**



**For: Stantec**

**November 5, 2018**

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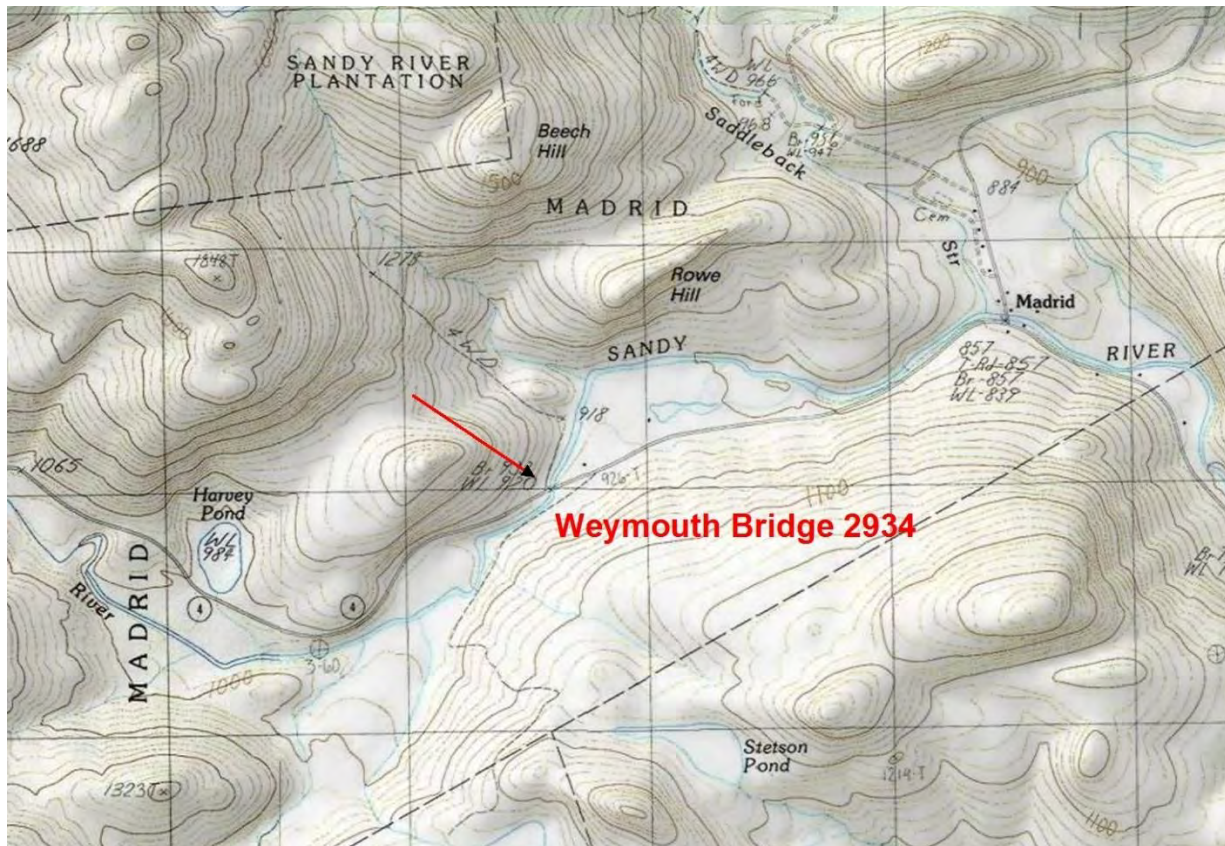
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# Final Design Hydrology and Hydraulics Report

The Weymouth Bridge in Madrid carries Route 4 over the Sandy River about 0.5 mile southeast of the Madrid Town Line. Figure 1 shows the site location.



The 45' single span bridge crosses a segment of river with rapid flows and steep gradient. The bridge crosses the river at approximately a 30-35 degree angle and Hurricane Irene in 2011 carved a scour hole downstream of the bridge. Figure 2 is an air photo that shows the bridge site.

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Final Design H-H Scour Report Madrid  
NHI WIN 22615.00

Stantec furnished plans dated February of 2017, and for the 90% submittal planned for May 14, 2018 showing the planned bridge profile and plan (Sheets 4 and 5).

Figure 2 shows the bridge on an aerial photo.



*Figure 2. Air photo of bridge crossing showing scour hole downstream. Note skew of bridge structure to direction of flow.*

The river segment contains a sharp bend and rapids upstream and a scour hole downstream. The left or westerly abutment projects into the river flow. Figure 3 is taken from the easterly upstream bank looking at the upstream western abutment face.



*Figure 3. Looking upstream. Note steep gradient and bend upstream.*

Figure 4 below is taken from Stantec's proposed plans of February, 2018 showing restored channel, bridge spanning natural channel, and preliminary design flood elevations. This same section is shown in the 90% submittal plans.

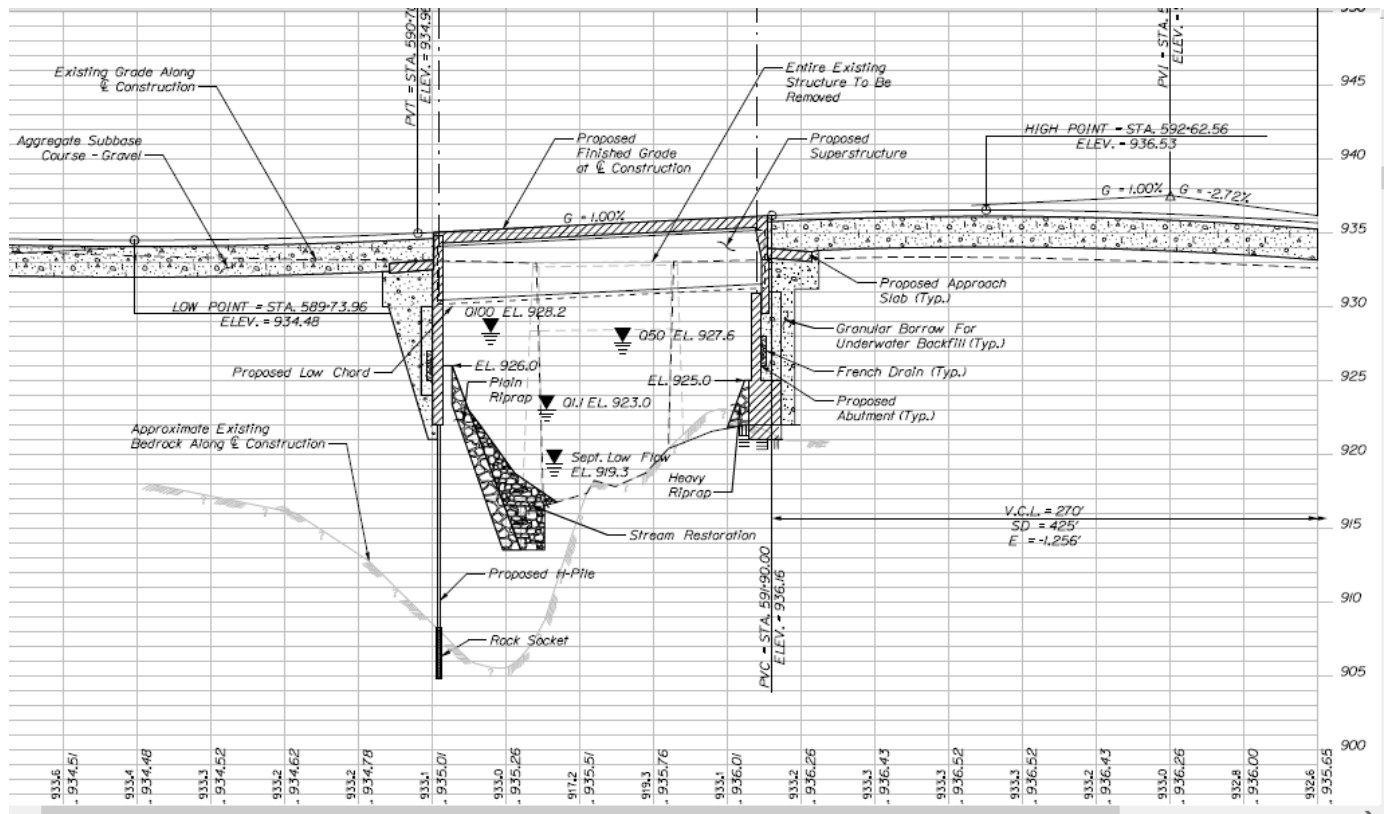


Figure 4. Proposed replacement bridge.

## 2.0 Existing Data Review

Existing data related to this bridge was reviewed, including but not limited to

- MDOT file information including
  - In 2010, a Bridge Scour Assessment Summary Report was issued by Maine DOT. The report is included in the appendix. The report notes concerns with ongoing scour on the left (westerly) abutment as well as a scour hole downstream. The bridge is rated “3” scour critical and scour protection is recommended for the left abutment. Note that this report was done prior to Hurricane Irene.
  - Structure Inventory and Appraisal Sheets from 2014 note scour as “4” stable but needs action.
- Historical Flood Information – No site specific flood information was found for this location. However, the 1987 flood in this area is estimated to be approximately a 150-year event. The 1936 hurricane is the next largest flood on the Sandy River. Information on Hurricane Irene in 2011 was recorded at Madrid (see below), and is likely the flood of record at this site.
- Surveyed channel cross sections and additional project survey were utilized for hydraulic modeling.

- Soil borings – material, depth, size and distribution of bottom materials.
- USGS gaging station records- the nearest long term USGS Gage on the Sandy River is near Mercer, Maine, about 40 miles downstream with a drainage area of 516 square miles. This gage record runs from 1928 to present (2017).

[http://waterdata.usgs.gov/nwis/inventory/?site\\_no=01048000&agency\\_cd=USGS](http://waterdata.usgs.gov/nwis/inventory/?site_no=01048000&agency_cd=USGS)

A short term gage is located on the bridge, drains 25.3 square miles and has records from 2008 to present. Flood of record at this gage was 5200 cfs during Hurricane Irene on August 28, 2011. Other annual peaks have ranged from 1500 to 2800 cfs.

[http://waterdata.usgs.gov/nwis/inventory/?site\\_no=01047200&agency\\_cd=USGS](http://waterdata.usgs.gov/nwis/inventory/?site_no=01047200&agency_cd=USGS)



Figure 5. Left or westerly abutment with stream gage installation on downstream wing.

- FEMA Flood Maps and FIS report
  - FEMA has not studied this area in detail. The Flood Insurance Rate Map is of the “approximate study” type, published in 1985. Flood zone is listed as “A” with no flood elevations shown.
- Air photos
- Topographic Maps

### 3.0 Hydrologic Analysis

Maine DOT furnished peak discharges for floods of selected design intervals, including the 1.1-year, 10-, 25-, 50-, 100-, and 500-year flood based on the USGS Regression Equations. At the Weymouth Bridge, drainage area is 25.3 square miles and percent wetlands is 2.6. Table 1 below summarizes peak flood flows as computed by Maine DOT and as used for this bridge analysis. In addition to flow data calculated by Maine DOT, the record for the USGS gage at the bridge was used to identify a range of typical annual peak flows and the flow for Hurricane Irene. The flow data for specific frequencies were calculated by the Regression formula. Irene and peak flows are as recorded by the gage.

No specific historic data was found for the 1987 for 1936 floods. However, recent Hurricane Irene (August 28, 2011) dropped 8.5 in. of rain on the Carrabassett Valley area, washing out two bridges on Route 27 near the Sugarloaf Access Road. This storm was recorded by the gage on the Sandy River at Madrid. For the 25 sq mi. mile drainage basin, the 5,200 cfs recorded flow is about a 500-year event.

**Summary of Available Hydrologic Data, Weymouth Bridge 2934, Madrid, Maine**

Recurrence Interval	Maine DOT calculations at gage
Calculation Method	25.3 sq mi, 2.6 % wetlands
1.1	535
2	1,127
5	1,796
10	2,304
25	2,992
50	3,540
100	4,124
500	5,604
Hurricane Irene (~500 yr)	5,200
Gage Annual Peak Low	1,500
Gage Annual Peak High	2,800
September Mean Low	5.8
October Mean Low	9.9

**Table 1.** Flow frequency data for recurrence intervals, calculated by Maine DOT. Hurricane Irene, annual flows and mean flows recorded by the USGS gage.

<b>Drainage Area</b>	<b>25 sq mi.</b>
Q25	2,992 cfs
Design Discharge (Q50)	3,540 cfs
Check Discharge (Q100)	4,124 cfs
Scour Check Discharge (Q500)	5,604 cfs
Ordinary High Water (Q1.1)	535 cfs
Flood of Record (Hurricane Irene)	5,200 cfs

**Table 2. Summary of Hydrologic Data for Bridge Analysis**

**Site Photographs:** Figures 6-14 are photographs relative to site hydraulics.



*Figure 6. Looking upstream from bridge. Note bend in river just upstream of bridge.*



*Figure 7. Looking at west upstream abutment. Note that abutment encroaches into main channel.*



*Figure 8. Looking downstream from bridge at scour hole.*



*Figure 9. Right or eastern abutment set on rock.*



*Figure 10. West abutment. Note cracking. Maine DOT inspection photos note cracking and rotation of abutment.*



*Figure 11. Looking downstream through bridge. Easterly or right abutment. Note scour hole downstream. Abutment founded on rock.*



*Figure 12. Right/East abutment looking upstream. Note abutment founded on rock.*



*Figure 13. Downstream left/westerly abutment. Note abutment projects into flow.*



*Figure 14. Upstream end of left/westerly abutment. Note projection into stream.*

#### **4.0 Hydraulic Analysis**

The goal of the hydraulic analysis was to provide information on flood elevations and flow velocities under existing and proposed conditions. Model data was used by the design team to establish design parameters for a replacement bridge including: required alternative road profiles, structure depths, and substructure layout. The hydraulic model was also used to assess channel conditions related to fish passage.

Figure 15 shows project survey and layout of model cross sections used for the hydraulic analysis. These sections and modified sections were used to simulate channel conditions as the bridge exists now and under expected conditions for a replacement bridge.

Note that for the final design model, sections at each face of the proposed bridge were added to the model skewed 35 degrees to simulate the area perpendicular to stream flow. This section layout was somewhat different than the preliminary design hydraulic model due to final design

geometry. The modeled bridge reach has conditions of rapidly varying flow and the bridge is not perpendicular to flow. This layout results in challenges for 1-dimensional flow model simulation to perfectly capture bridge hydraulics. HECRAS calculates both water surface elevation and energy grade line, providing additional information to designers on expected water levels under flood conditions.



Figure 15. Channel cross sections.

The existing bridge crosses the river at approximately a 35 degree angle. While the existing bridge span is 48 ft, the channel width from abutment to abutment perpendicular to flow is approximately 40 ft. A large scour hole was created downstream of the bridge by flows

generated by Hurricane Irene. The existing abutments project somewhat into the natural channel. The channel has rapidly varying flow conditions, with a drop into the scour pool.

The hydraulic analysis calculates flood levels such that the bridge may be designed for passage of peak flood flows with sufficient clearance for selected flood events (typically 2 ft at Q50 for minor structures such as this bridge, 4 ft for major bridges). The analysis also provided variables used for projecting scour depths. In addition, pre- and post- condition flood elevations were determined and compared to assure that flood levels will not be raised by the new bridge. The hydraulic analysis also assisted with determining average Bank Full Width channel section for design related to fish passage.

Hydraulic analysis was performed in accordance with the procedures outlined in the Maine DOT Bridge Design Guide. Hydraulic analysis was prepared using HECRAS, version 5.03 River Analysis System by the U.S. Army Corps of Engineers.

Modeled cross sections are shown in Figure 15 above. In addition to sections shown on this plan, two new sections at the bridge faces for each design option were included in the final model, and skewed 35 degrees to plot perpendicular to flow.

The proposed bridge layout is shown in Figure 16. The width of the proposed bridge perpendicular to flow increases from approximately 40 ft to 85 ft.

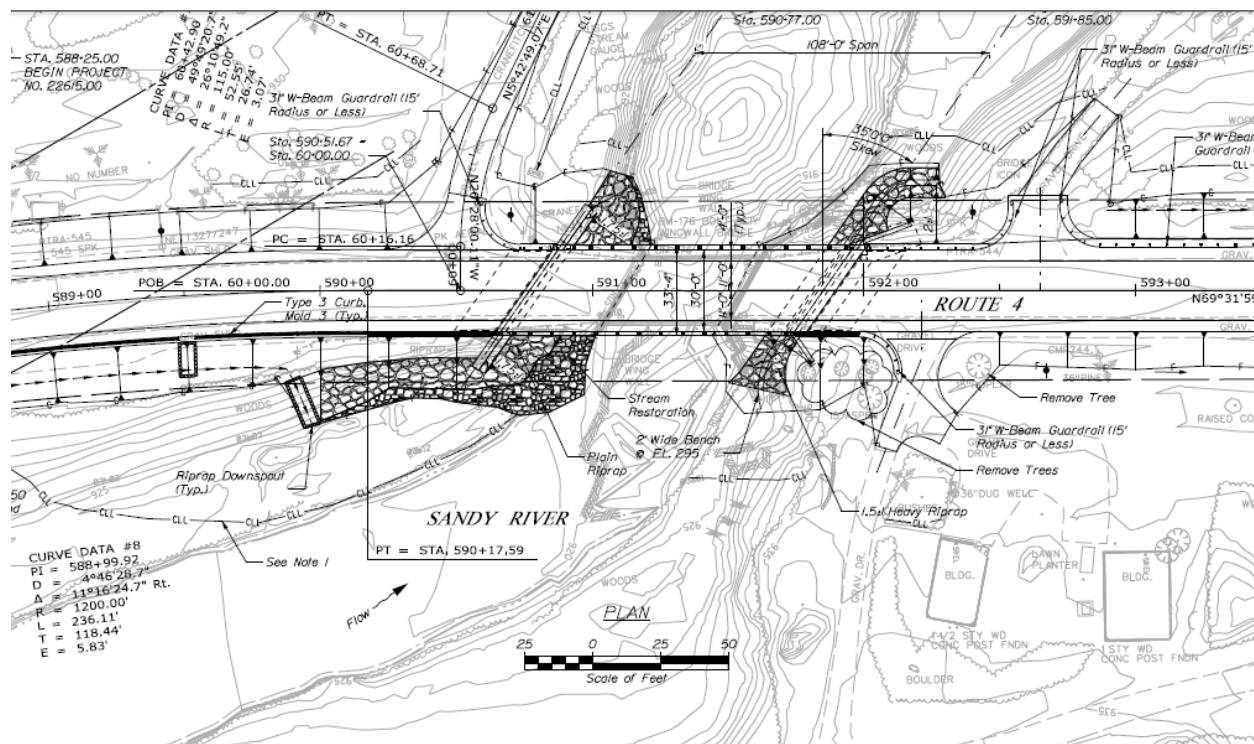


Figure 16. Final design bridge layout

The expanded width of the proposed bridge allows for improved flood passage. Figure 17 shows a comparison of the flood elevations under the existing condition (blue) and the proposed bridge (red). The narrower opening of the existing bridge caused backwater upstream of the bridge and

high velocities through the bridge. The wider proposed opening allows flows to pass without significant upstream impact and will mitigate scour by lowering discharge velocities.

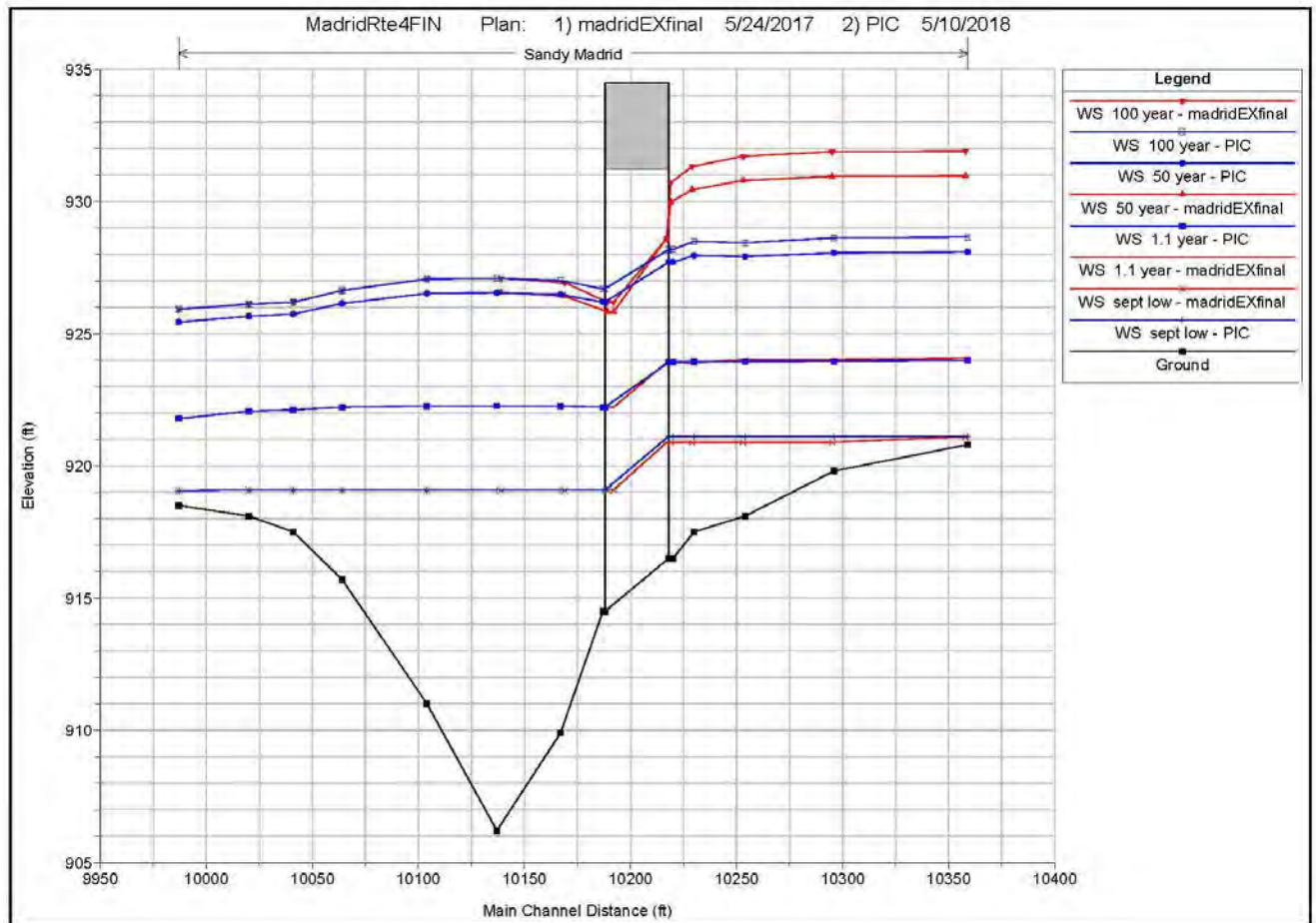


Figure 17. Channel bottom and existing and proposed flood profiles. September low flow, 1.1-year flow, 50- and 100-year flows.

Table 3 below summarizes hydraulic data for existing and proposed conditions.

Summary of Hydraulic Data	Existing Bridge	Proposed Bridge
Low Chord	928.6	930.1 (L) -931.2 (R)
Width at Q100 (perpendicular to flow)	39.7	85.3
Width at Banks (perpendicular to flow)	39.5	84.7
Headwater at Q25, ft, (at bridge face)	927.3	927.2
Energy Grade Line for Q25 at US face.	928.4	927.7
Headwater at Q50, ft, (at bridge face)	928.6	927.7
Energy Grade Line for Q50 at US face	930.9	928.3
Headwater at Q100, ft, (at bridge face)	928.6	928.2
Energy Grade Line for Q100 at US face	931.8	928.9
Discharge Velocity at Q25, fps	8.9	6.2
Discharge Velocity at Q50, fps	10.1	6.7
Discharge Velocity at Q100, fps	11.3	7.2
Ordinary High Water Elevation (Q1.1) (US face), ft	923.9	923.9
Discharge Velocity at Q1.1, fps	2.5	2.2
Clearance @ Q25, ft	0.2	2.9-4.0
Clearance @ Q50, ft	EGL 2.3 above low cord, pressure flow	2.4-3.5
Clearance @ Q100, ft	EGL 3.2 above low chord, pressure flow	1.9-3.0
Bridge Opening Area, ft <sup>2</sup>	400.5	864.7
Flow Area at Q100, ft <sup>2</sup> (US Face)	400.5	629.1

**Table 3. Summary of Hydraulic Data.**

## 5.0 Scour Computations

The existing bridge shows evidence of scour by contraction (scour hole) and at the west abutment. The right or easterly abutment will be founded on rock. The west or left abutment will require scour protection. Contraction scour potential decreases significantly due to the proposed bridge section being wider than bank full width. Because abutments will not be subject to scour due to presence of rock or scour protection, only contraction scour was computed.

Scour analysis for the proposed 104 ft span at a thirty five degree angle to flow yielded the following results:

Proposed Bridge	100-year		500-year	
	Left	center	left	center
D50 (assumed)	1mm		1 mm	
Contraction Scour	0.0'	0.80	0.0'	1.4'
Abutment Scour Depth	N/A scour protection		NA scour protection	
Total Scour	0.0	0.8'	0.0'	1.4'
Channel Elev. Before scour	921.9	916.0	921.9	916.0
Est. Channel Elevation after scour	921.9	915.2	921.9	914.6

**Table 4. Scour Calculations**

The project site was checked for susceptibility to rock scour, but rock scour is generally thought to apply only to scour at piers. Critical stream power was computed based on logged rock qualities, but calculation of actual stream power can only be done at piers.

### Fish Passage:

The Sandy River flows into the Kennebec River and is thus part of the Atlantic Salmon Restoration Initiative. Improved fish passage allowance is required for the new bridge. The existing bridge constricts the natural channel as do the old abutments downstream. Bank full width was estimated to be 73 ft perpendicular to the channel based on several estimation methods, including examination of surveyed sections, calculations with HECRAS, sketches of main channel banks and field examination. Detail is included in the appendix. For the proposed bridge design including proposed channel reconstruction, channel width at the 1.1-yr event will be 69 ft perpendicular to the channel. Note that this design has a total bridge span of 104 ft which translates to an approximately 85 foot span abutment to abutment in the direction perpendicular to flow. Sloped embankments, bedrock and scour protection make the channel narrower at the 1.1-year flood level.

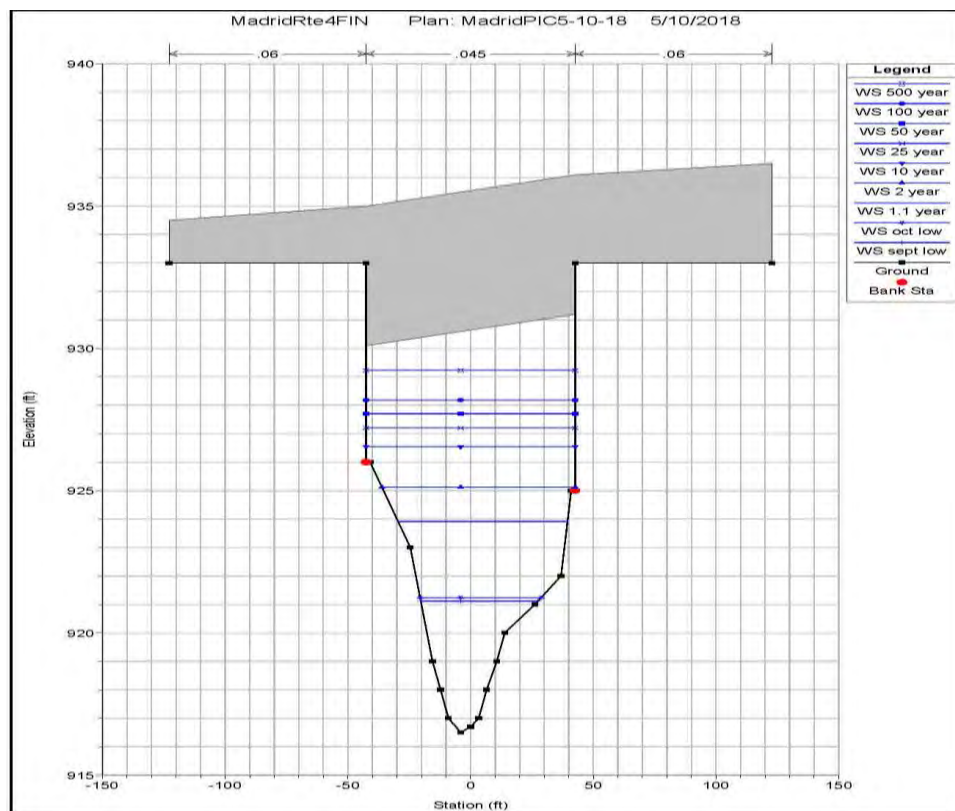


Figure 18. Bridge cross section, HECRAS model. – US section.

## 6.0 Summary of Findings

- The bridge crosses the channel at an approximately 35 degree angle.
- The existing bridge constricts the channel to narrower than natural bank full width.

- The Flood Insurance Study does not provide a 100-year elevation at this location.
- Model results indicate that in a 50-year flood event, the existing bridge is likely to have flood water above the low chord.
- The drainage area is 25.3 square miles and a short term recording gage installed and maintained by USGS is located on the downstream side of the bridge.
- The hydraulic characteristics of the bridge site indicate rapid flow upstream of and through the bridge. The stream flows into a scour pool below the bridge.
- Starting water levels for the hydraulic model were based on calculated stream gradient and normal depth.
- For the proposed bridge, 50-year clearance is 2.4-3.5 ft, and 100-year clearance is predicted to be 1.9-3.0 ft according to the hydraulic model.
- The East abutment is to be founded on rock, and the west abutment is subject to scour unless protected with scour protection. The west abutment will be protected with scour protection.
- Predicted contraction scour is limited to less than 1.1' for the new bridge due to the wider span. Rock scour is limited to piers, so is not a factor at this site.
- Improvements in fish passage will be provided by widening the bridge opening to slightly narrower than bank full width, or 69 ft perpendicular to flow direction.
- The wider bridge will result in less likelihood of backwater during flood events.

## References

Federal Emergency Management Agency. Flood Insurance Rate Map, Town of Madrid, Maine. 1985

U. S. Dept. of the Interior, Geological Survey, with Maine Dept. of Transportation. Estimating the Magnitude and Frequency of Peak Flows for Streams in Maine for Selected Recurrence Intervals. Water Resources Investigations Report 99-4008.

U.S. Army Corps of Engineers, Hydrologic Engineering Center. HEC-RAS River Analysis System. Version 5.0 Beta. January, 2010. Davis, CA

U.S. Department of Transportation. Federal Highway Administration. Evaluating Scour at Bridges, 5<sup>th</sup> edition. HEC-18. April 2012, Publication No. FHWA-HIF-12-003

U.S. Department of Transportation. Federal Highway Administration. Bridge Scour and Stream Instability Countermeasures. HEC-23. Volume 2. September, 2009. FHWA-NHI-09-112 . DG 14, Rock Riprap at Bridge Abutments and DG 4, Riprap Revetment

Maine Dept. of Transportation. Bridge Design Manual. August 2003

ESRI ArcMap, ArcGIS Desktop, Version 9.3.1, 2009. Arcview license. Data added from MEGIS website, project plans and ESRI

[http://www.theirregular.com/news/2011-08-31/Front\\_Page/Irenes\\_comings\\_and\\_goings.html](http://www.theirregular.com/news/2011-08-31/Front_Page/Irenes_comings_and_goings.html)

Maine Dept. of Transportation. Structure Inventory and Appraisal Sheet, Bridge # 2934 3/18/2014

Maine Dept. of Transportation. Bridge Scour Assessment Summary Report. Weymouth Bridge, Madrid, Maine. Bridge # 2934. 1/29/10

USGS Gage on the Sandy River near Mercer, Maine, # 01048000  
[http://waterdata.usgs.gov/nwis/inventory/?site\\_no=01048000&agency\\_cd=USGS](http://waterdata.usgs.gov/nwis/inventory/?site_no=01048000&agency_cd=USGS)

USGS Gage on Sandy River near Madrid, Maine #  
[http://waterdata.usgs.gov/nwis/inventory/?site\\_no=01047200&agency\\_cd=USGS](http://waterdata.usgs.gov/nwis/inventory/?site_no=01047200&agency_cd=USGS)

Stantec for MaineDOT,. Madrid Franklin County, Weymouth Bridge over Sandy River. Route 4 Federal Aid Project No. STP-226(500), Bridge Number 2934. 90% Submission. 5/14/2018

**Weymouth Bridge, Sandy River, Madrid, Maine**  
**Final Design Hydrologic and Hydraulics Report**

**Appendix**

Hydrology Calculations	Pages 1-3
Hydraulic Model Output	
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Bridge Scour Assessment Summary Report	Pages 39-40
Bridge Scour Calculations	Pages 41

Project Name:	Madrid - Weymouth Br
Stream Name:	Sandy River
Bridge Name:	Weymouth Bridge
Route No.	ME 4
Analysis by:	CSH

PIN:	22615
Town:	Madrid
Bridge No.	2934
USGS Quad:	
Date:	3/19/2014

## Peak Flow Calculations by USGS Regression Equations (Hodgkins, 1999)

Enter data in blue cells only!

	km <sup>2</sup>	mi <sup>2</sup>	ac
A	65.79	25.40	16256.0
W	1.71	0.66	423.0
P <sub>c</sub>	378070	4968047	
County	Franklin		
pptA	45.6		
SG	0.00		
A (km <sup>2</sup> )	65.79		
W (%)	2.60		
			Conf Lvl
			0.67

Enter data in [mi<sup>2</sup>]

Watershed Area  
Wetlands area (by NWI)

watershed centroid (E, N; UTM 19N; meters)  
choose county from drop-down menu  
mean annual precipitation (inches; by look-up)  
sand & gravel aquifer as decimal fraction of watershed A

Worksheet prepared by:  
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Maine Dept. Transportation  
Augusta, ME 04333-0016  
207-557-1052  
[Charles.Hebson@maine.gov](mailto:Charles.Hebson@maine.gov)

Ret Pd	Peak Flow Estimate	Lower	Q <sub>T</sub> (m <sup>3</sup> /s)	Upper
T (yr)				
1.1		15.16		
2		22.75	31.91	44.76
5		36.08	50.86	71.71
10		45.86	65.24	92.81
25		58.70	84.72	122.28
50		68.64	100.26	146.44
100		78.97	116.78	172.70
500		103.89	158.70	242.45

Q<sub>T</sub> (ft<sup>3</sup>/s)

535.4
1126.9
1796.0
2303.6
2991.6
3540.1
4123.6
5603.8

Reference:

Hodgkins, G., 1999.  
Estimating the magnitude of peak flows for streams  
in Maine for selected recurrence intervals  
*Water-Resources Investigations Report 99-4008*  
US Geological Survey, Augusta, Maine

$$Q_T = b \times A^a \times 10^{-wW}$$

## Log-Normal Probability Plot



Project Name:	Madrid - Weymouth Br
Stream Name:	Sandy River
Bridge Name:	Weymouth Bridge
Route No.	ME 4
Analysis by:	CSH

PIN:	22615
Town:	Madrid
Bridge No.	2934
USGS Quad:	
Date:	3/19/2014

**DO NOT ENTER ANY DATA ON THIS PAGE; EVERYTHING IS CALCULATED**

**MAINE MONTHLY MEDIAN FLOWS BY USGS REGRESSION EQUATIONS (2004)**

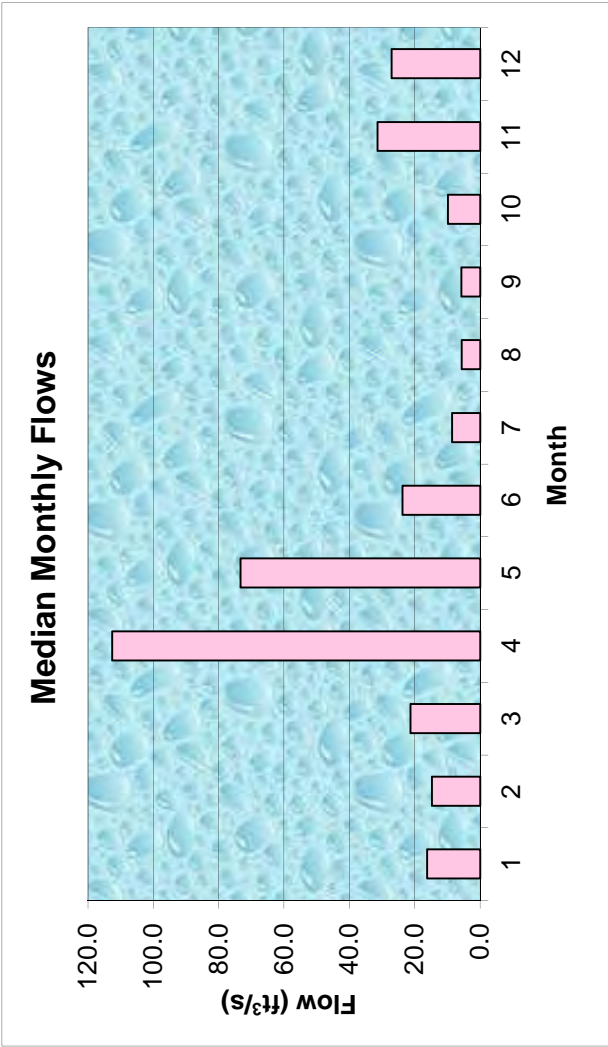
Value	Variable	Explanation
25.400	A	Area (mi <sup>2</sup> )
378070	P <sub>c</sub>	Watershed centroid (E,N; UTM; Zone 19; meters)
4968047	DIST	Distance from Coastal reference line (mi)
111.68	pptA	Mean Annual Precipitation (inches)
45.6	SG	Sand & Gravel Aquifer (decimal fraction of watershed area)
0.00		

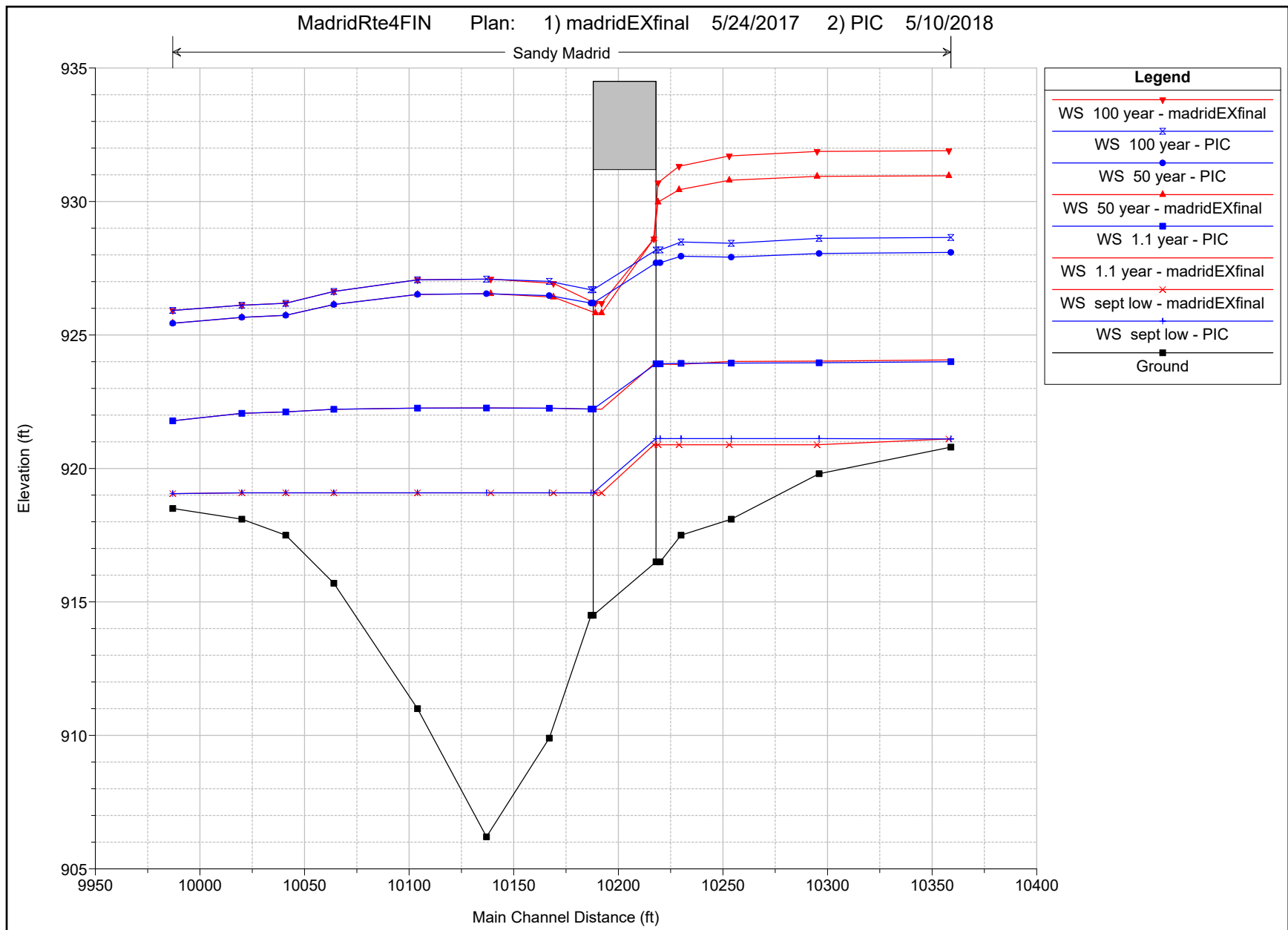
**Worksheet prepared by:**  
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Augusta, ME 04333-0016  
207-624-3073  
[Charles.Hebson@maine.gov](mailto:Charles.Hebson@maine.gov)

Month	Q <sub>median</sub> (ft <sup>3</sup> /s)	(m <sup>3</sup> /s)
Jan	16.26	0.4607
Feb	14.75	0.4180
Mar	21.32	0.6041
Apr	112.64	3.1922
May	73.38	2.0795
Jun	23.84	0.6756
Jul	8.64	0.2448
Aug	5.69	0.1613
Sep	5.80	0.1643
Oct	9.91	0.2807
Nov	31.44	0.8911
Dec	27.13	0.7688

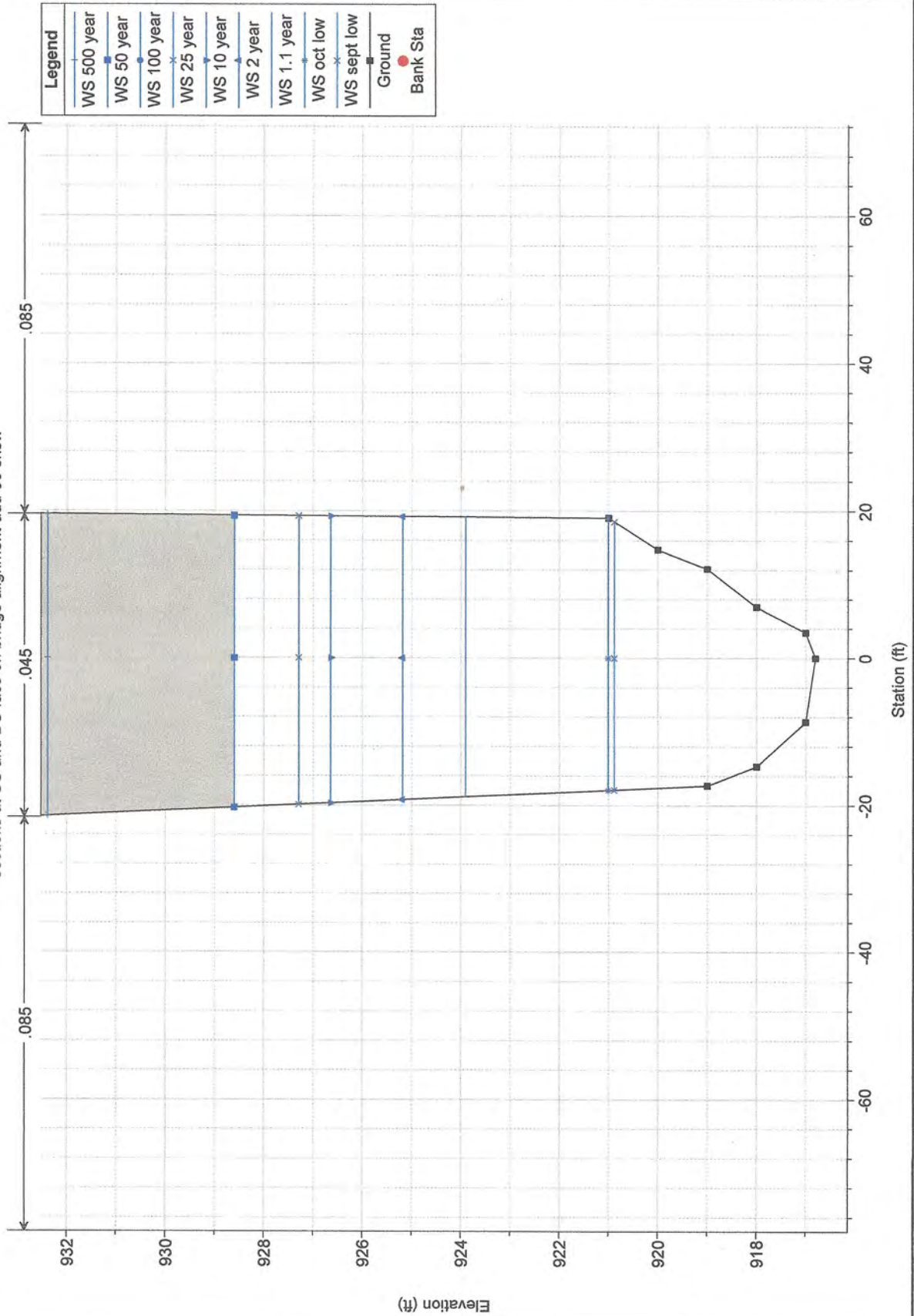
Q <sub>bf</sub>	155.0
ann avg	51.9
ann med	27.6
Q <sub>1,002</sub>	226.3
Q <sub>1,01</sub>	307.3
Q <sub>1,05</sub>	444.4

W <sub>bf</sub>	41.0
d <sub>bf</sub>	3.2
Q <sub>bf</sub>	523.4 assume v = 4ft/s

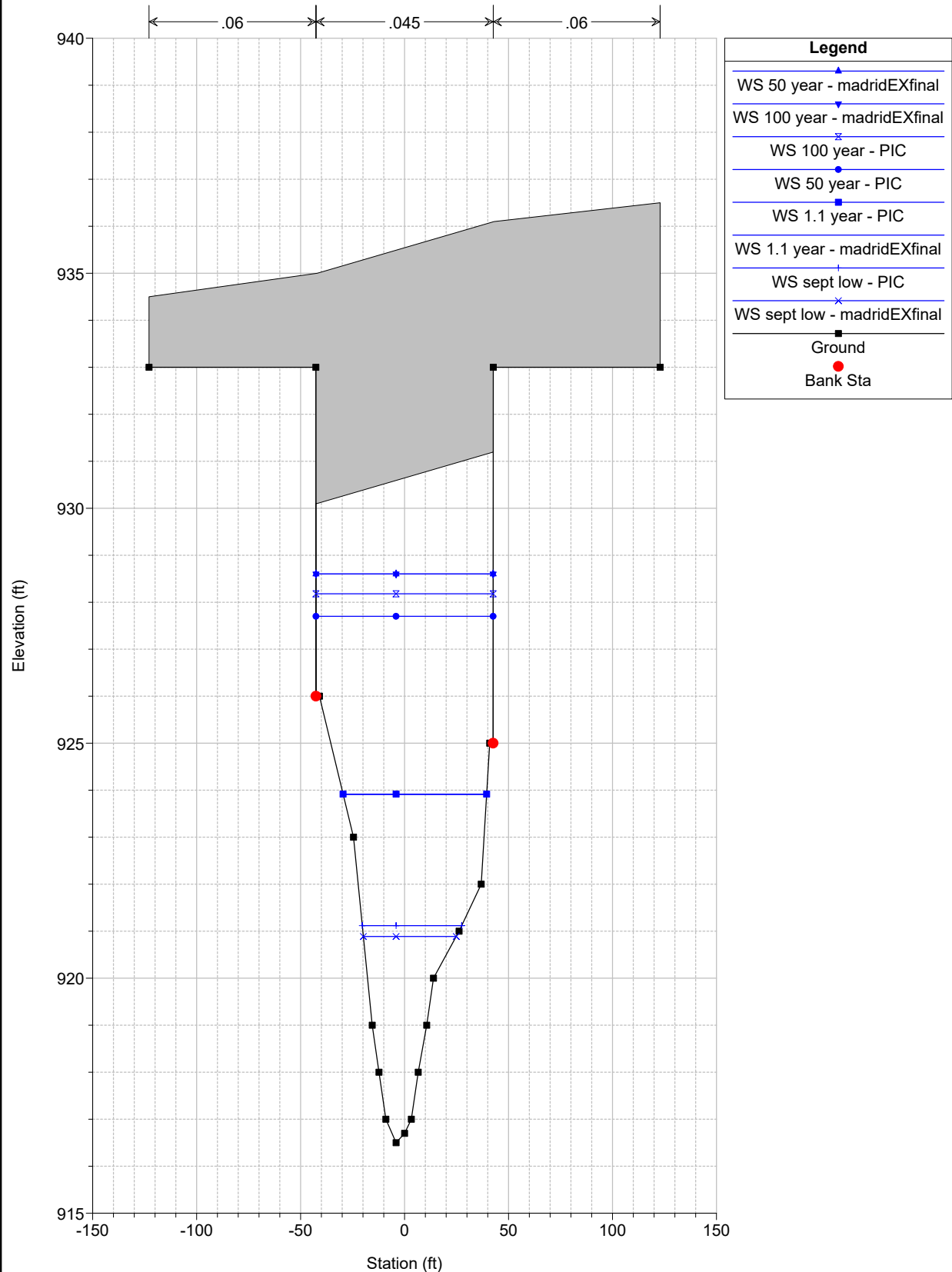


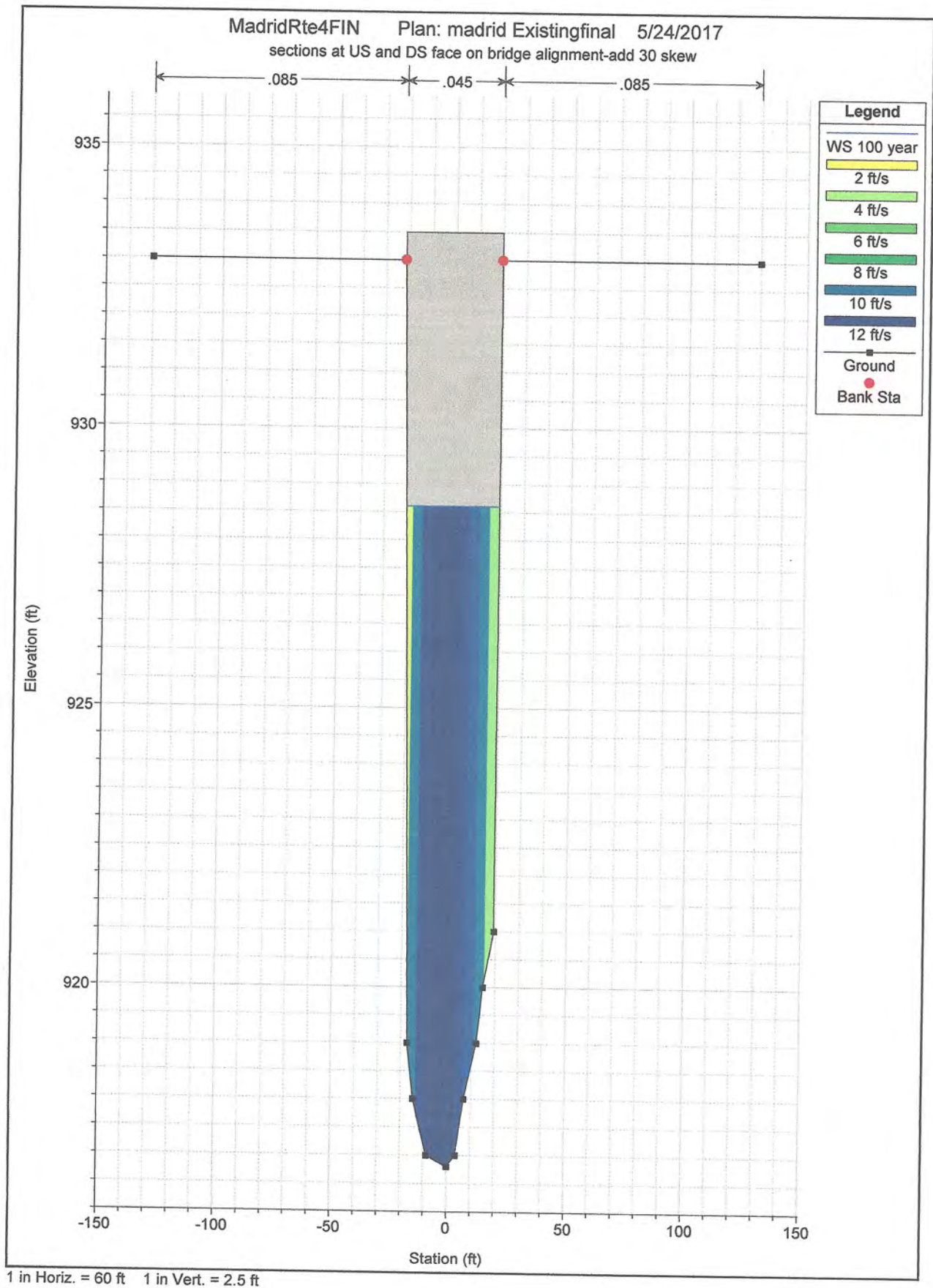


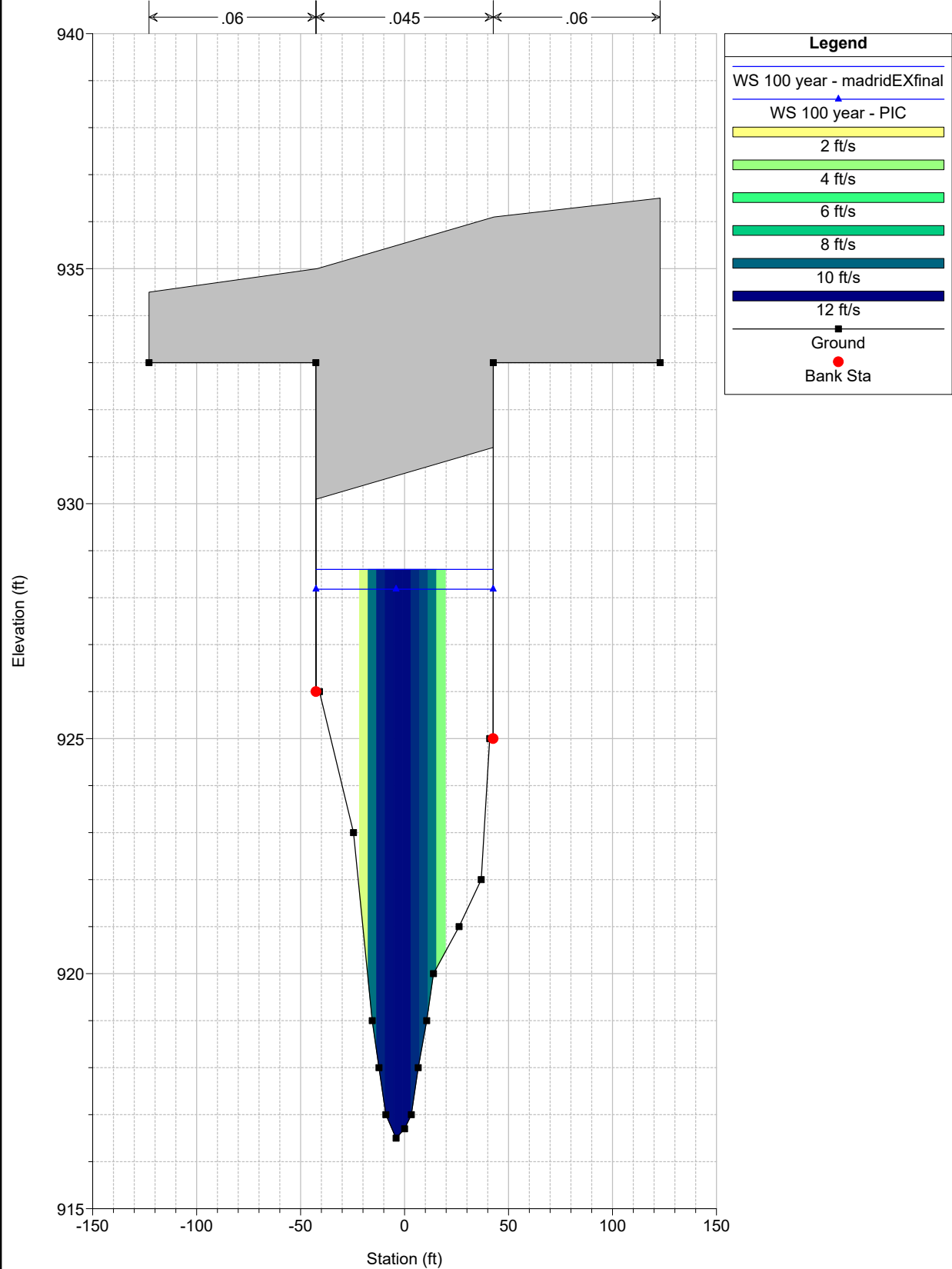
MadridRte4FIN Plan: madrid Existingfinal 5/24/2017  
sections at US and DS face on bridge alignment-add 30 skew



MadridRte4FIN Plan: 1) madridEXfinal 5/24/2017 2) PIC 5/10/2018









HEC-RAS Plan: PIC River: Sandy Reach: Madrid (Continued)

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Madrid	10136	500 year	5604.00	906.20	928.28		928.55	0.000408	4.25	1500.43	170.91	0.18
Madrid	10136	sept low	5.80	906.20	919.09		919.09	0.000000	0.01	589.07	79.98	0.00
Madrid	10136	oct low	9.90	906.20	919.22		919.22	0.000000	0.02	599.78	80.35	0.00
Madrid	10103	1.1 year	535.00	911.00	922.26		922.27	0.000035	0.76	729.10	109.45	0.05
Madrid	10103	2 year	1127.00	911.00	923.61		923.64	0.000093	1.36	878.54	111.56	0.08
Madrid	10103	10 year	2304.00	911.00	925.22		925.30	0.000232	2.34	1059.30	114.08	0.12
Madrid	10103	25 year	2992.00	911.00	925.96		926.08	0.000316	2.83	1145.00	116.69	0.15
Madrid	10103	50 year	3540.00	911.00	926.52		926.67	0.000386	3.19	1211.48	125.16	0.16
Madrid	10103	100 year	4124.00	911.00	927.06		927.25	0.000455	3.53	1286.57	145.93	0.18
Madrid	10103	500 year	5604.00	911.00	928.26		928.54	0.000605	4.34	1482.56	199.45	0.21
Madrid	10103	sept low	5.80	911.00	919.09		919.09	0.000000	0.01	442.19	78.59	0.00
Madrid	10103	oct low	9.90	911.00	919.22		919.22	0.000000	0.02	452.72	79.05	0.00
Madrid	10064	1.1 year	535.00	915.70	922.21		922.26	0.000374	1.78	320.35	84.58	0.14
Madrid	10064	2 year	1127.00	915.70	923.50		923.62	0.000722	2.88	456.04	116.70	0.20
Madrid	10064	10 year	2304.00	915.70	924.97		925.26	0.001341	4.52	631.56	122.55	0.29
Madrid	10064	25 year	2992.00	915.70	925.63		926.03	0.001642	5.29	714.19	125.05	0.32
Madrid	10064	50 year	3540.00	915.70	926.14		926.61	0.001834	5.82	778.00	127.08	0.35
Madrid	10064	100 year	4124.00	915.70	926.63		927.18	0.002029	6.34	841.90	150.37	0.37
Madrid	10064	500 year	5604.00	915.70	927.70		928.45	0.002447	7.49	1035.76	190.44	0.41
Madrid	10064	sept low	5.80	915.70	919.09		919.09	0.000001	0.05	120.33	51.25	0.01
Madrid	10064	oct low	9.90	915.70	919.22		919.22	0.000002	0.08	127.20	51.73	0.01
Madrid	10041	1.1 year	535.00	917.50	922.12		922.24	0.001280	2.87	211.23	90.26	0.26
Madrid	10041	2 year	1127.00	917.50	923.31		923.58	0.002070	4.37	339.94	134.03	0.34
Madrid	10041	10 year	2304.00	917.50	924.66		925.19	0.003170	6.35	524.51	138.54	0.44
Madrid	10041	25 year	2992.00	917.50	925.27		925.94	0.003686	7.27	610.05	144.46	0.49
Madrid	10041	50 year	3540.00	917.50	925.73		926.52	0.004004	7.91	679.07	154.09	0.51
Madrid	10041	100 year	4124.00	917.50	926.19		927.08	0.004297	8.52	751.06	164.86	0.54
Madrid	10041	500 year	5604.00	917.50	927.22		928.33	0.004758	9.74	958.48	239.01	0.57
Madrid	10041	sept low	5.80	917.50	919.09		919.09	0.000014	0.14	42.28	35.51	0.02
Madrid	10041	oct low	9.90	917.50	919.22		919.22	0.000030	0.21	47.10	36.91	0.03
Madrid	10020	1.1 year	535.00	918.10	922.07		922.21	0.001982	3.08	202.80	89.06	0.31
Madrid	10020	2 year	1127.00	918.10	923.23		923.53	0.002875	4.59	319.55	132.33	0.39
Madrid	10020	10 year	2304.00	918.10	924.57		925.11	0.003956	6.45	499.85	137.00	0.48
Madrid	10020	25 year	2992.00	918.10	925.19		925.86	0.004381	7.28	585.13	139.30	0.51
Madrid	10020	50 year	3540.00	918.10	925.66		926.42	0.004585	7.82	651.99	144.27	0.53
Madrid	10020	100 year	4124.00	918.10	926.11		926.98	0.004808	8.37	719.38	152.27	0.55
Madrid	10020	500 year	5604.00	918.10	927.11		928.22	0.005352	9.63	900.47	213.90	0.59
Madrid	10020	sept low	5.80	918.10	919.09		919.09	0.000135	0.29	19.95	30.09	0.06
Madrid	10020	oct low	9.90	918.10	919.22		919.22	0.000235	0.41	24.03	32.63	0.08
Madrid	9987	1.1 year	535.00	918.50	921.79	920.98	922.08	0.006009	4.42	130.00	70.83	0.51
Madrid	9987	2 year	1127.00	918.50	922.91	921.95	923.38	0.006001	5.76	264.62	148.74	0.55
Madrid	9987	10 year	2304.00	918.50	924.30	923.44	924.94	0.006007	7.22	475.16	155.59	0.58
Madrid	9987	25 year	2992.00	918.50	924.95	923.94	925.68	0.005999	7.86	577.82	160.13	0.59
Madrid	9987	50 year	3540.00	918.50	925.44	924.30	926.25	0.006003	8.32	658.34	169.64	0.60
Madrid	9987	100 year	4124.00	918.50	925.92	924.63	926.80	0.006002	8.76	742.43	179.55	0.61
Madrid	9987	500 year	5604.00	918.50	926.98	925.37	928.01	0.006003	9.70	961.18	232.11	0.62
Madrid	9987	sept low	5.80	918.50	919.05	918.82	919.07	0.006002	1.08	5.35	19.30	0.36
Madrid	9987	oct low	9.90	918.50	919.17	918.92	919.20	0.005999	1.28	7.75	21.78	0.38

HEC-RAS Plan: PIC River: Sandy Reach: Madrid

Reach	River Sta	Profile	E.G. Elev (ft)	W.S. Elev (ft)	Crit W.S. (ft)	Frctn Loss (ft)	C & E Loss (ft)	Top Width (ft)	Q Left (cfs)	Q Channel (cfs)	Q Right (cfs)	Vel Chnl (ft/s)
Madrid	10233	1.1 year	923.98	923.94		0.00	0.00	101.30	2.33	526.12	6.55	1.68
Madrid	10233	2 year	925.28	925.17		0.01	0.01	116.94	11.64	1081.71	33.65	2.67
Madrid	10233	10 year	926.94	926.69		0.02	0.03	122.43	44.18	2133.65	126.18	4.13
Madrid	10233	25 year	927.73	927.41		0.02	0.04	122.62	71.65	2730.32	190.03	4.79
Madrid	10233	50 year	928.34	927.95		0.02	0.05	122.77	95.75	3199.22	245.03	5.24
Madrid	10233	100 year	928.94	928.48		0.02	0.06	122.91	122.44	3695.89	305.67	5.69
Madrid	10233	500 year	930.30	929.68		0.03	0.10	123.22	192.78	4945.50	465.72	6.70
Madrid	10233	sept low	921.12	921.12		0.00	0.00	67.82		5.80		0.05
Madrid	10233	oct low	921.24	921.24		0.00	0.00	68.70		9.90		0.09
Madrid	10221	1.1 year	923.97	923.92	919.81			69.07		535.00		1.89
Madrid	10221	2 year	925.26	925.12	921.33			78.73		1127.00		3.03
Madrid	10221	10 year	926.89	926.55	922.90			85.21	0.00	2304.00	0.00	4.70
Madrid	10221	25 year	927.67	927.21	923.62			85.23	0.00	2992.00	0.00	5.48
Madrid	10221	50 year	928.27	927.70	924.15			85.24	0.00	3540.00	0.00	6.02
Madrid	10221	100 year	928.85	928.19	924.67			85.25	0.00	4124.00	0.00	6.55
Madrid	10221	500 year	930.18	929.24	925.86			85.27	0.00	5603.99	0.01	7.79
Madrid	10221	sept low	921.12	921.12	916.89			47.80		5.80		0.05
Madrid	10221	oct low	921.24	921.24	917.00			49.41		9.90		0.08
Madrid	10200 BR U	1.1 year	923.97	923.92	919.83			69.06		535.00		1.89
Madrid	10200 BR U	2 year	925.26	925.12	921.34			78.73		1127.00		3.03
Madrid	10200 BR U	10 year	926.89	926.54	922.92			85.21	0.00	2304.00	0.00	4.71
Madrid	10200 BR U	25 year	927.67	927.20	923.65			85.23	0.00	2992.00	0.00	5.48
Madrid	10200 BR U	50 year	928.26	927.70	924.19			85.24	0.00	3540.00	0.00	6.02
Madrid	10200 BR U	100 year	928.85	928.18	924.71			85.25	0.00	4123.99	0.01	6.56
Madrid	10200 BR U	500 year	930.17	929.23	925.87			85.27	0.01	5603.98	0.01	7.80
Madrid	10200 BR U	sept low	921.12	921.12	916.90			47.80		5.80		0.05
Madrid	10200 BR U	oct low	921.24	921.24	917.00			49.41		9.90		0.08
Madrid	10200 BR D	1.1 year	922.30	922.22	917.73			59.10		535.00		2.15
Madrid	10200 BR D	2 year	923.70	923.52	919.16			64.67		1127.00		3.43
Madrid	10200 BR D	10 year	925.45	925.01	921.46			77.44		2304.00		5.33
Madrid	10200 BR D	25 year	926.27	925.68	922.31			81.62		2992.00	0.00	6.15
Madrid	10200 BR D	50 year	926.89	926.20	922.86			85.21		3540.00	0.00	6.68
Madrid	10200 BR D	100 year	927.51	926.70	923.44			85.22	0.00	4124.00	0.00	7.21
Madrid	10200 BR D	500 year	928.87	927.76	924.81			85.24	0.00	5603.99	0.01	8.46
Madrid	10200 BR D	sept low	919.09	919.09	914.91			34.41		5.80		0.05
Madrid	10200 BR D	oct low	919.22	919.22	915.01			34.73		9.90		0.09
Madrid	10197	1.1 year	922.29	922.22		0.00	0.02	59.10		535.00		2.15
Madrid	10197	2 year	923.70	923.52		0.01	0.04	64.66		1127.00		3.43
Madrid	10197	10 year	925.44	925.00		0.02	0.09	77.42		2304.00		5.33
Madrid	10197	25 year	926.27	925.68		0.03	0.12	81.59		2992.00	0.00	6.15
Madrid	10197	50 year	926.89	926.20		0.03	0.13	85.21		3540.00	0.00	6.69
Madrid	10197	100 year	927.50	926.69		0.03	0.15	85.22	0.00	4124.00	0.00	7.21
Madrid	10197	500 year	928.87	927.75		0.04	0.19	85.24	0.00	5604.00	0.00	8.47
Madrid	10197	sept low	919.09	919.09		0.00	0.00	34.41		5.80		0.05
Madrid	10197	oct low	919.22	919.22		0.00	0.00	34.73		9.90		0.09
Madrid	10166	1.1 year	922.27	922.26		0.00	0.00	82.96	1.21	533.79		1.03
Madrid	10166	2 year	923.65	923.60		0.00	0.01	85.98	4.27	1122.73		1.79
Madrid	10166	10 year	925.33	925.19		0.01	0.02	91.28	13.49	2290.51		3.03
Madrid	10166	25 year	926.13	925.92		0.01	0.03	91.95	20.23	2971.77	0.00	3.63
Madrid	10166	50 year	926.73	926.48		0.01	0.04	92.46	26.38	3513.62	0.00	4.06
Madrid	10166	100 year	927.32	927.01		0.01	0.04	92.95	33.52	4090.48	0.00	4.49
Madrid	10166	500 year	928.63	928.17		0.02	0.06	102.62	53.92	5550.08	0.00	5.49
Madrid	10166	sept low	919.09	919.09		0.00	0.00	59.93	0.00	5.80		0.02
Madrid	10166	oct low	919.22	919.22		0.00	0.00	60.82	0.00	9.90		0.03

# Proposed

Plan: PIC Sandy Madrid RS: 10200 Profile: 100 year

E.G. US. (ft)	928.85	Element	Inside BR US	Inside BR DS
W.S. US. (ft)	928.19	E.G. Elev (ft)	928.85	927.51
Q Total (cfs)	4124.00	W.S. Elev (ft)	928.18	926.70
Q Bridge (cfs)	4124.00	Crit W.S. (ft)	924.71	923.44
Q Weir (cfs)		Max Chl Dpth (ft)	11.68	12.20
Weir Sta Lft (ft)		Vel Total (ft/s)	6.56	7.21
Weir Sta Rgt (ft)		Flow Area (sq ft)	629.09	572.01
Weir Submerg		Froude # Chl	0.43	0.49
Weir Max Depth (ft)		Specif Force (cu ft)	3505.00	3443.20
Min El Weir Flow (ft)	934.51	Hydr Depth (ft)	7.38	6.71
Min El Prs (ft)	931.20	W.P. Total (ft)	93.50	93.74
Delta EG (ft)	1.35	Conv. Total (cfs)	76989.2	64166.9
Delta WS (ft)	1.49	Top Width (ft)	85.25	85.22
BR Open Area (sq ft)	839.13	Frctn Loss (ft)		
BR Open Vel (ft/s)	7.21	C & E Loss (ft)		
BR Sluice Coef		Shear Total (lb/sq ft)	1.21	1.57
BR Sel Method	Momentum	Power Total (lb/ft s)	7.90	11.35

# Proposed

Plan: PIC Sandy Madrid RS: 10200 BR U Profile: 100 year

	Pos	Left Sta	Right Sta	Flow	Area	W.P.	Percent	Hydr	Velocity	Shear	Power
		(ft)	(ft)	(cfs)	(sq ft)	(ft)	Conv	Depth(ft)	(ft/s)	(lb/sq ft)	(lb/ft s)
1	LOB	-122.87	-42.60	0.00	0.03	2.18	0.00	1.09	0.07	0.00	0.00
2	Chan	-42.60	-34.08	71.77	22.91	8.63	1.74	2.69	3.13	0.48	1.49
3	Chan	-34.08	-25.56	151.78	35.96	8.66	3.68	4.22	4.22	0.74	3.14
4	Chan	-25.56	-17.04	310.02	56.65	9.24	7.52	6.65	5.47	1.10	6.01
5	Chan	-17.04	-8.52	625.88	85.28	8.96	15.18	10.01	7.34	1.71	12.51
6	Chan	-8.52	0.00	815.76	98.10	8.55	19.78	11.52	8.32	2.06	17.10
7	Chan	0.00	8.52	717.73	91.67	8.74	17.40	10.76	7.83	1.88	14.71
8	Chan	8.52	17.04	498.00	73.62	8.74	12.08	8.64	6.76	1.51	10.21
9	Chan	17.04	25.56	406.36	64.58	8.55	9.85	7.58	6.29	1.35	8.52
10	Chan	25.56	34.08	342.29	58.29	8.56	8.30	6.84	5.87	1.22	7.17
11	Chan	34.08	42.60	184.41	41.96	9.51	4.47	4.93	4.40	0.79	3.47
12	ROB	42.60	122.87	0.01	0.05	3.18	0.00	1.59	0.11	0.00	0.00

Plan: PIC Sandy Madrid RS: 10200 BR U Profile: 500 year

	Pos	Left Sta	Right Sta	Flow	Area	W.P.	Percent	Hydr	Velocity	Shear	Power
		(ft)	(ft)	(cfs)	(sq ft)	(ft)	Conv	Depth(ft)	(ft/s)	(lb/sq ft)	(lb/ft s)
1	LOB	-122.87	-42.60	0.01	0.06	3.23	0.00	1.61	0.10	0.00	0.00
2	Chan	-42.60	-34.08	137.68	31.84	8.63	2.46	3.74	4.32	0.78	3.39
3	Chan	-34.08	-25.56	243.51	44.88	8.66	4.35	5.27	5.43	1.10	5.98
4	Chan	-25.56	-17.04	438.63	65.58	9.24	7.83	7.70	6.69	1.51	10.08
5	Chan	-17.04	-8.52	819.14	94.20	8.96	14.62	11.06	8.70	2.23	19.43
6	Chan	-8.52	0.00	1045.69	107.03	8.55	18.66	12.56	9.77	2.66	26.00
7	Chan	0.00	8.52	929.02	100.59	8.74	16.58	11.81	9.24	2.45	22.59
8	Chan	8.52	17.04	668.14	82.55	8.74	11.92	9.69	8.09	2.01	16.25
9	Chan	17.04	25.56	559.01	73.51	8.55	9.98	8.63	7.60	1.83	13.90
10	Chan	25.56	34.08	481.21	67.21	8.56	8.59	7.89	7.16	1.67	11.95
11	Chan	34.08	42.60	281.97	50.88	9.51	5.03	5.97	5.54	1.14	6.30
12	ROB	42.60	122.87	0.01	0.09	4.23	0.00	2.11	0.15	0.00	0.00

Plan: PIC Sandy Madrid RS: 10200 BR D Profile: 100 year

	Pos	Left Sta	Right Sta	Flow	Area	W.P.	Percent	Hydr	Velocity	Shear	Power
		(ft)	(ft)	(cfs)	(sq ft)	(ft)	Conv	Depth(ft)	(ft/s)	(lb/sq ft)	(lb/ft s)
1	LOB	-122.87	-42.60	0.00	0.00	0.70	0.00	0.35	0.04	0.00	0.00
2	Chan	-42.60	-34.08	19.21	9.79	8.61	0.47	1.15	1.96	0.29	0.58
3	Chan	-34.08	-25.56	70.52	21.39	8.63	1.71	2.51	3.30	0.64	2.11
4	Chan	-25.56	-17.04	234.89	50.37	12.08	5.70	5.91	4.66	1.07	5.01
5	Chan	-17.04	-8.52	918.58	101.26	8.95	22.27	11.89	9.07	2.92	26.46
6	Chan	-8.52	0.00	917.03	99.73	8.64	22.24	11.71	9.20	2.98	27.37
7	Chan	0.00	8.52	774.09	90.04	8.63	18.77	10.57	8.60	2.69	23.13
8	Chan	8.52	17.04	503.35	71.23	9.16	12.21	8.36	7.07	2.00	14.17
9	Chan	17.04	25.56	321.44	52.97	8.56	7.79	6.22	6.07	1.60	9.68
10	Chan	25.56	34.08	252.99	45.88	8.56	6.13	5.39	5.51	1.38	7.62
11	Chan	34.08	42.60	111.90	29.34	9.51	2.71	3.44	3.81	0.80	3.03
12	ROB	42.60	122.87	0.00	0.01	1.70	0.00	0.85	0.09	0.00	0.00

Plan: PIC Sandy Madrid RS: 10200 BR D Profile: 500 year

	Pos	Left Sta	Right Sta	Flow	Area	W.P.	Percent	Hydr	Velocity	Shear	Power
		(ft)	(ft)	(cfs)	(sq ft)	(ft)	Conv	Depth(ft)	(ft/s)	(lb/sq ft)	(lb/ft s)
1	LOB	-122.87	-42.60	0.00	0.02	1.76	0.00	0.88	0.08	0.00	0.00
2	Chan	-42.60	-34.08	63.16	18.83	8.61	1.13	2.21	3.35	0.64	2.14
3	Chan	-34.08	-25.56	140.29	30.43	8.63	2.50	3.57	4.61	1.03	4.75
4	Chan	-25.56	-17.04	341.91	59.40	12.08	6.10	6.97	5.76	1.44	8.26
5	Chan	-17.04	-8.52	1171.11	110.30	8.95	20.90	12.95	10.62	3.60	38.19
6	Chan	-8.52	0.00	1171.58	108.77	8.64	20.91	12.77	10.77	3.68	39.59
7	Chan	0.00	8.52	1003.75	99.08	8.63	17.91	11.63	10.13	3.35	33.97
8	Chan	8.52	17.04	679.09	80.27	9.16	12.12	9.42	8.46	2.56	21.64
9	Chan	17.04	25.56	462.09	62.00	8.56	8.25	7.28	7.45	2.12	15.76
10	Chan	25.56	34.08	377.45	54.92	8.56	6.74	6.45	6.87	1.87	12.88

Plan: PIC Sandy Madrid RS: 10200 BR D Profile: 500 year (Continued)

	Pos	Left Sta	Right Sta	Flow	Area	W.P.	Percent	Hydr	Velocity	Shear	Power
		(ft)	(ft)	(cfs)	(sq ft)	(ft)	Conv	Depth(ft)	(ft/s)	(lb/sq ft)	(lb/ft s)
11	Chan	34.08	42.60	193.56	38.37	9.51	3.45	4.50	5.04	1.18	5.94
12	ROB	42.60	122.87	0.01	0.04	2.76	0.00	1.38	0.13	0.00	0.00



HEC-RAS Plan: madridEXfinal River: Sandy Reach: Madrid (Continued)

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chi
Madrid	10136	50 year	3540.00	906.20	926.55		926.68	0.000237	3.01	1259.70	103.84	0.14
Madrid	10136	100 year	4124.00	906.20	927.09		927.27	0.000286	3.39	1319.16	114.73	0.15
Madrid	10136	500 year	5604.00	906.20	928.28		928.56	0.000413	4.27	1472.62	165.95	0.18
Madrid	10136	sept low	5.80	906.20	919.09		919.09	0.000000	0.01	588.86	79.51	0.00
Madrid	10136	oct low	9.90	906.20	919.22		919.22	0.000000	0.02	599.47	79.81	0.00
Madrid	10103	1.1 year	535.00	911.00	922.26		922.27	0.000035	0.76	729.13	109.45	0.05
Madrid	10103	2 year	1127.00	911.00	923.61		923.64	0.000093	1.35	878.60	111.56	0.08
Madrid	10103	10 year	2304.00	911.00	925.22		925.30	0.000232	2.34	1059.47	114.09	0.12
Madrid	10103	25 year	2992.00	911.00	925.95		926.08	0.000316	2.83	1145.26	116.70	0.15
Madrid	10103	50 year	3540.00	911.00	926.52		926.67	0.000386	3.19	1211.67	125.24	0.16
Madrid	10103	100 year	4124.00	911.00	927.07		927.25	0.000454	3.53	1286.95	145.99	0.18
Madrid	10103	500 year	5604.00	911.00	928.26		928.54	0.000605	4.34	1483.08	199.52	0.21
Madrid	10103	sept low	5.80	911.00	919.09		919.09	0.000000	0.01	442.22	78.59	0.00
Madrid	10103	oct low	9.90	911.00	919.22		919.22	0.000000	0.02	452.72	79.05	0.00
Madrid	10084	1.1 year	535.00	915.70	922.21		922.26	0.000374	1.78	320.37	84.59	0.14
Madrid	10084	2 year	1127.00	915.70	923.50		923.62	0.000722	2.88	456.11	116.70	0.20
Madrid	10084	10 year	2304.00	915.70	924.97		925.26	0.001340	4.52	631.74	122.55	0.29
Madrid	10084	25 year	2992.00	915.70	925.64		926.03	0.001640	5.29	714.48	125.06	0.32
Madrid	10084	50 year	3540.00	915.70	926.14		926.61	0.001833	5.82	778.18	127.09	0.35
Madrid	10084	100 year	4124.00	915.70	926.63		927.18	0.002026	6.34	842.30	150.81	0.37
Madrid	10084	500 year	5604.00	915.70	927.70		928.45	0.002445	7.49	1036.25	190.46	0.41
Madrid	10084	sept low	5.80	915.70	919.09		919.09	0.000001	0.05	120.35	51.26	0.01
Madrid	10084	oct low	9.90	915.70	919.22		919.22	0.000002	0.08	127.20	51.73	0.01
Madrid	10041	1.1 year	535.00	917.50	922.12		922.24	0.001280	2.87	211.25	90.27	0.26
Madrid	10041	2 year	1127.00	917.50	923.31		923.58	0.002069	4.37	340.03	134.03	0.34
Madrid	10041	10 year	2304.00	917.50	924.66		925.19	0.003166	6.34	524.78	138.55	0.44
Madrid	10041	25 year	2992.00	917.50	925.27		925.95	0.003680	7.27	610.46	144.51	0.49
Madrid	10041	50 year	3540.00	917.50	925.74		926.52	0.004000	7.91	679.36	154.14	0.51
Madrid	10041	100 year	4124.00	917.50	926.19		927.08	0.004289	8.52	751.65	164.94	0.53
Madrid	10041	500 year	5604.00	917.50	927.22		928.33	0.004747	9.73	959.53	239.05	0.57
Madrid	10041	sept low	5.80	917.50	919.09		919.09	0.000014	0.14	42.29	35.52	0.02
Madrid	10041	oct low	9.90	917.50	919.22		919.22	0.000030	0.21	47.10	36.91	0.03
Madrid	10020	1.1 year	535.00	918.10	922.07		922.21	0.001982	3.08	202.80	89.06	0.31
Madrid	10020	2 year	1127.00	918.10	923.23		923.53	0.002875	4.59	319.56	132.33	0.39
Madrid	10020	10 year	2304.00	918.10	924.57		925.11	0.003955	6.45	499.85	137.00	0.48
Madrid	10020	25 year	2992.00	918.10	925.19		925.86	0.004380	7.28	585.15	139.30	0.51
Madrid	10020	50 year	3540.00	918.10	925.66		926.42	0.004589	7.82	651.76	144.25	0.53
Madrid	10020	100 year	4124.00	918.10	926.11		926.98	0.004809	8.37	719.33	152.27	0.55
Madrid	10020	500 year	5604.00	918.10	927.12		928.22	0.005330	9.62	904.20	214.12	0.59
Madrid	10020	sept low	5.80	918.10	919.09		919.09	0.000135	0.29	19.97	30.10	0.06
Madrid	10020	oct low	9.90	918.10	919.22		919.22	0.000235	0.41	24.03	32.63	0.08
Madrid	9987	1.1 year	535.00	918.50	921.79	920.98	922.08	0.006009	4.42	130.00	70.83	0.51
Madrid	9987	2 year	1127.00	918.50	922.91	921.95	923.38	0.006001	5.76	264.62	148.74	0.55
Madrid	9987	10 year	2304.00	918.50	924.30	923.44	924.94	0.006007	7.22	475.16	155.59	0.58
Madrid	9987	25 year	2992.00	918.50	924.95	923.94	925.68	0.005999	7.66	577.82	160.13	0.59
Madrid	9987	50 year	3540.00	918.50	925.44	924.30	926.25	0.006003	8.32	658.34	169.64	0.60
Madrid	9987	100 year	4124.00	918.50	925.92	924.83	926.80	0.006002	8.76	742.43	179.55	0.61
Madrid	9987	500 year	5604.00	918.50	926.98	925.37	928.01	0.006003	9.70	961.18	232.11	0.62
Madrid	9987	sept low	5.80	918.50	919.05	918.82	919.07	0.006002	1.08	5.35	19.30	0.36
Madrid	9987	oct low	9.90	918.50	919.17	918.92	919.20	0.005999	1.28	7.75	21.78	0.38

# Existing

HEC-RAS Plan: madridEXfinal River: Sandy Reach: Madrid

Reach	River Sta	Profile	E.G. Elev (ft)	W.S. Elev (ft)	Crit W.S. (ft)	Frctn Loss (ft)	C & E Loss (ft)	Top Width (ft)	Q Left (cfs)	Q Channel (cfs)	Q Right (cfs)	Vel Chnl (ft/s)
Madrid	10233	1.1 year	924.04	923.90		0.01	0.02	52.09		531.27	3.73	3.01
Madrid	10233	2 year	925.52	925.19		0.02	0.03	59.27		1108.22	18.78	4.70
Madrid	10233	10 year	927.49	926.70		0.04	0.00	73.09		2219.11	84.89	7.26
Madrid	10233	25 year	928.55	927.56		0.06	0.04	77.26		2828.23	163.77	8.19
Madrid	10233	50 year	931.05	930.44		0.03	0.10	86.87		3154.20	385.80	6.59
Madrid	10233	100 year	932.00	931.32		0.03	0.13	93.10		3628.52	495.48	6.98
Madrid	10233	500 year	934.26	933.45		0.04	0.24	183.17	15.38	4779.85	808.77	7.72
Madrid	10233	sept low	920.89	920.89		0.00	0.00	32.57		5.80		0.14
Madrid	10233	oct low	921.01	921.01		0.00	0.00	44.19		9.90		0.22
Madrid	10222	1.1 year	924.00	923.91	919.71			38.02		535.00		2.45
Madrid	10222	2 year	925.47	925.20	921.11			38.47		1127.00		4.21
Madrid	10222	10 year	927.44	926.66	923.00			38.99		2304.00		7.11
Madrid	10222	25 year	928.45	927.31	923.91			39.22		2992.00		8.56
Madrid	10222	50 year	930.92	929.98	924.61			40.16		3540.00		7.77
Madrid	10222	100 year	931.83	930.70	925.31			40.41		4124.00		8.51
Madrid	10222	500 year	933.98	932.39	926.98			41.01		5604.00		10.13
Madrid	10222	sept low	920.89	920.89	917.09			36.47		5.80		0.06
Madrid	10222	oct low	921.01	921.01	917.17			37.00		9.90		0.09
Madrid	10200 BR U	1.1 year	924.00	923.91	919.71			38.02		535.00		2.45
Madrid	10200 BR U	2 year	925.47	925.19	921.12			38.47		1127.00		4.22
Madrid	10200 BR U	10 year	927.43	926.64	923.00			38.98		2304.00		7.12
Madrid	10200 BR U	25 year	928.44	927.29	923.96			39.21		2992.00		8.57
Madrid	10200 BR U	50 year	930.92	928.60	924.63			39.67		3540.00		8.84
Madrid	10200 BR U	100 year	931.83	928.60	925.35			39.67		4124.00		10.30
Madrid	10200 BR U	500 year	933.98	932.39	926.98				273.56	5042.97	278.81	12.00
Madrid	10200 BR U	sept low	920.89	920.89	917.09			36.47		5.80		0.06
Madrid	10200 BR U	oct low	921.01	921.01	917.17			37.00		9.90		0.09
Madrid	10200 BR D	1.1 year	922.32	922.22	917.89			37.65		535.00		2.52
Madrid	10200 BR D	2 year	923.79	923.50	919.46			38.02		1127.00		4.33
Madrid	10200 BR D	10 year	925.72	924.87	921.47			38.40		2304.00		7.38
Madrid	10200 BR D	25 year	926.69	925.45	922.39			38.57		2992.00		8.93
Madrid	10200 BR D	50 year	927.42	925.83	923.11			38.67		3540.00		10.13
Madrid	10200 BR D	100 year	928.19	926.19	923.82			38.78		4124.00		11.34
Madrid	10200 BR D	500 year	933.90	932.39	925.46				273.56	5042.97	278.81	10.56
Madrid	10200 BR D	sept low	919.09	919.09	915.17			32.89		5.80		0.06
Madrid	10200 BR D	oct low	919.22	919.22	915.24			33.29		9.90		0.10
Madrid	10197	1.1 year	922.32	922.22		0.00	0.04	37.65		535.00		2.52
Madrid	10197	2 year	923.78	923.49		0.01	0.12	38.01		1127.00		4.33
Madrid	10197	10 year	925.70	924.86		0.02	0.33	38.40		2304.00		7.38
Madrid	10197	25 year	926.67	925.42		0.03	0.49	38.56		2992.00		8.96
Madrid	10197	50 year	927.42	925.83		0.04	0.63	38.67		3540.00		10.13
Madrid	10197	100 year	928.19	926.19		0.04	0.79	38.78		4124.00		11.34
Madrid	10197	500 year	930.05	926.79	925.45	0.06	1.30	38.95		5604.00		14.47
Madrid	10197	sept low	919.09	919.09		0.00	0.00	32.89		5.80		0.06
Madrid	10197	oct low	919.22	919.22		0.00	0.00	33.29		9.90		0.10
Madrid	10166	1.1 year	922.28	922.26		0.00	0.00	64.97	1.11	533.87	0.02	1.08
Madrid	10166	2 year	923.66	923.60		0.00	0.01	66.36	4.07	1122.82	0.11	1.96
Madrid	10166	10 year	925.35	925.17		0.01	0.03	67.98	13.12	2290.48	0.40	3.42
Madrid	10166	25 year	926.15	925.88		0.01	0.05	68.72	20.10	2971.26	0.64	4.16
Madrid	10166	50 year	926.76	926.42		0.01	0.06	69.27	26.57	3512.56	0.87	4.70
Madrid	10166	100 year	927.36	926.93		0.01	0.08	69.81	34.16	4088.70	1.14	5.25
Madrid	10166	500 year	928.69	928.03		0.02	0.12	72.50	55.95	5546.11	1.94	6.56
Madrid	10166	sept low	919.09	919.09		0.00	0.00	59.93	0.00	5.80		0.02
Madrid	10166	oct low	919.22	919.22		0.00	0.00	60.82	0.00	9.90		0.03

Plan: madrid<sup>EX</sup>final Sandy Madrid RS: 10200 Profile: 100 year

E.G. US. (ft)	931.83	Element	Inside BR US	Inside BR DS
W.S. US. (ft)	930.70	E.G. Elev (ft)	931.83	928.19
Q Total (cfs)	4124.00	W.S. Elev (ft)	928.60	926.19
Q Bridge (cfs)	4124.00	Crit W.S. (ft)	925.35	923.82
Q Weir (cfs)		Max Chl Dpth (ft)	11.80	11.19
Weir Sta Lft (ft)		Vel Total (ft/s)	10.30	11.34
Weir Sta Rgt (ft)		Flow Area (sq ft)	400.54	363.56
Weir Submerg		Froude # Chl	0.57	0.65
Weir Max Depth (ft)		Specif Force (cu ft)	3433.43	3253.39
Min El Weir Flow (ft)	933.01	Hydr Depth (ft)	10.10	9.38
Min El Prs (ft)	928.60	W.P. Total (ft)	54.84	54.18
Delta EG (ft)	3.64	Conv. Total (cfs)	49787.5	42711.0
Delta WS (ft)	4.52	Top Width (ft)	39.67	38.78
BR Open Area (sq ft)	400.54	Frctn Loss (ft)		
BR Open Vel (ft/s)	10.30	C & E Loss (ft)		
BR Sluice Coef	0.45	Shear Total (lb/sq ft)	3.13	3.91
BR Sel Method	Press Only	Power Total (lb/ft s)	32.21	44.31

Plan: madridEXfinal Sandy Madrid RS: 10200 BR U Profile: 100 year

	Pos	Left Sta	Right Sta	Flow	Area	W.P.	Percent	Hydr	Velocity	Shear	Power
		(ft)	(ft)	(cfs)	(sq ft)	(ft)	Conv	Depth(ft)	(ft/s)	(lb/sq ft)	(lb/ft s)
1	Chan	-21.65	-17.53	33.98	12.33	9.35	0.82	4.46	2.76	0.57	1.56
2	Chan	-17.53	-13.41	411.80	42.26	4.82	9.99	10.25	9.74	3.75	36.59
3	Chan	-13.41	-9.28	521.63	45.99	4.18	12.65	11.16	11.34	4.72	53.48
4	Chan	-9.28	-5.16	562.89	47.93	4.13	13.65	11.63	11.74	4.97	58.36
5	Chan	-5.16	-1.04	571.90	48.35	4.12	13.87	11.73	11.83	5.02	59.41
6	Chan	-1.04	3.08	571.67	48.36	4.13	13.86	11.73	11.82	5.02	59.32
7	Chan	3.08	7.21	510.69	45.81	4.27	12.38	11.11	11.15	4.60	51.24
8	Chan	7.21	11.33	444.13	41.84	4.20	10.77	10.15	10.61	4.27	45.32
9	Chan	11.33	15.45	362.55	37.55	4.34	8.79	9.11	9.66	3.70	35.77
10	Chan	15.45	19.57	132.76	30.13	11.31	3.22	7.66	4.41	1.14	5.03

Plan: madridEXfinal Sandy Madrid RS: 10200 BR D Profile: 100 year

	Pos	Left Sta	Right Sta	Flow	Area	W.P.	Percent	Hydr	Velocity	Shear	Power
		(ft)	(ft)	(cfs)	(sq ft)	(ft)	Conv	Depth(ft)	(ft/s)	(lb/sq ft)	(lb/ft s)
1	Chan	-21.65	-17.58	107.49	25.50	12.77	2.61	8.26	4.22	1.16	4.90
2	Chan	-17.58	-13.51	605.51	45.53	4.07	14.68	11.19	13.30	6.51	86.58
3	Chan	-13.51	-9.44	605.51	45.53	4.07	14.68	11.19	13.30	6.51	86.58
4	Chan	-9.44	-5.37	605.51	45.53	4.07	14.68	11.19	13.30	6.51	86.58
5	Chan	-5.37	-1.30	581.94	44.68	4.12	14.11	10.98	13.02	6.31	82.18
6	Chan	-1.30	2.77	516.88	41.84	4.18	12.53	10.28	12.35	5.83	72.00
7	Chan	2.77	6.84	413.13	36.95	4.28	10.02	9.08	11.18	5.02	56.12
8	Chan	6.84	10.91	340.16	32.52	4.17	8.25	7.99	10.46	4.54	47.52
9	Chan	10.91	14.98	278.16	28.98	4.22	6.74	7.12	9.60	3.99	38.32
10	Chan	14.98	19.05	69.72	16.48	8.22	1.69	5.27	4.23	1.17	4.94

**Madrid: Weymouth Bridge**

Span of Current Bridge: 45 FT  
 Year of Construction: 1933  
 Type of Bridge: Concrete T Beam

**Madrid**

September-October Low Flows: 5.8 (SEPT) 9.9 (OCT) **Channel Type:**  
 Existing Flow Depth for Low Flows: 2.2-2.3' Incised Stream  
 Drop in water surface through bridge: 0.63' Rock Base  
 Maximum Velocity (low flow): 0.16-0.18 fps No chance of meandering

Bank Full Width				
Stream Cross Section Number	1. Width of Channel at time of Survey (FT)	2. Natural Channel Geomorphic Estimate (FT)	3. Cross Section Survey Plots (FT)	4. Hydraulic Model HECRAS 1.1-year flow width (FT)
99+87	45	75	53	71
100+20	50	75	53	67
100+41	50	75	55	67
100+64	54	75	68	60
101+03	82	75	87	109
101+36	80	75	85	87
101+66	60	75	62	65
Average each method	60.1	75.0	66.1	75.1
<b>AVERAGE WIDTH (FT): Downstream River Segment (Using Methods 2, 3, 4):</b>				<b>72</b>
Bridge	47/40			40
102+33				50
102+58	47	75	60	60
102+99	53	75	78	85
103+62	60	75	72	76
104+10	60	75		
104+60	60	75		
105+10	55	75		
105+60	50	75		
Average of each method	55	75	70.0	73.7
<b>AVERAGE WIDTH (FT): Upstream River Segment (Using Methods 2, 3, 4):</b>				<b>73</b>

Recommended Bank Full Width (Perpendicular to the channel edge): 73.0 FT

Bridge Length required for Bank Width Recommended (Skew = 35°): 90.0 FT

**METHODS:**

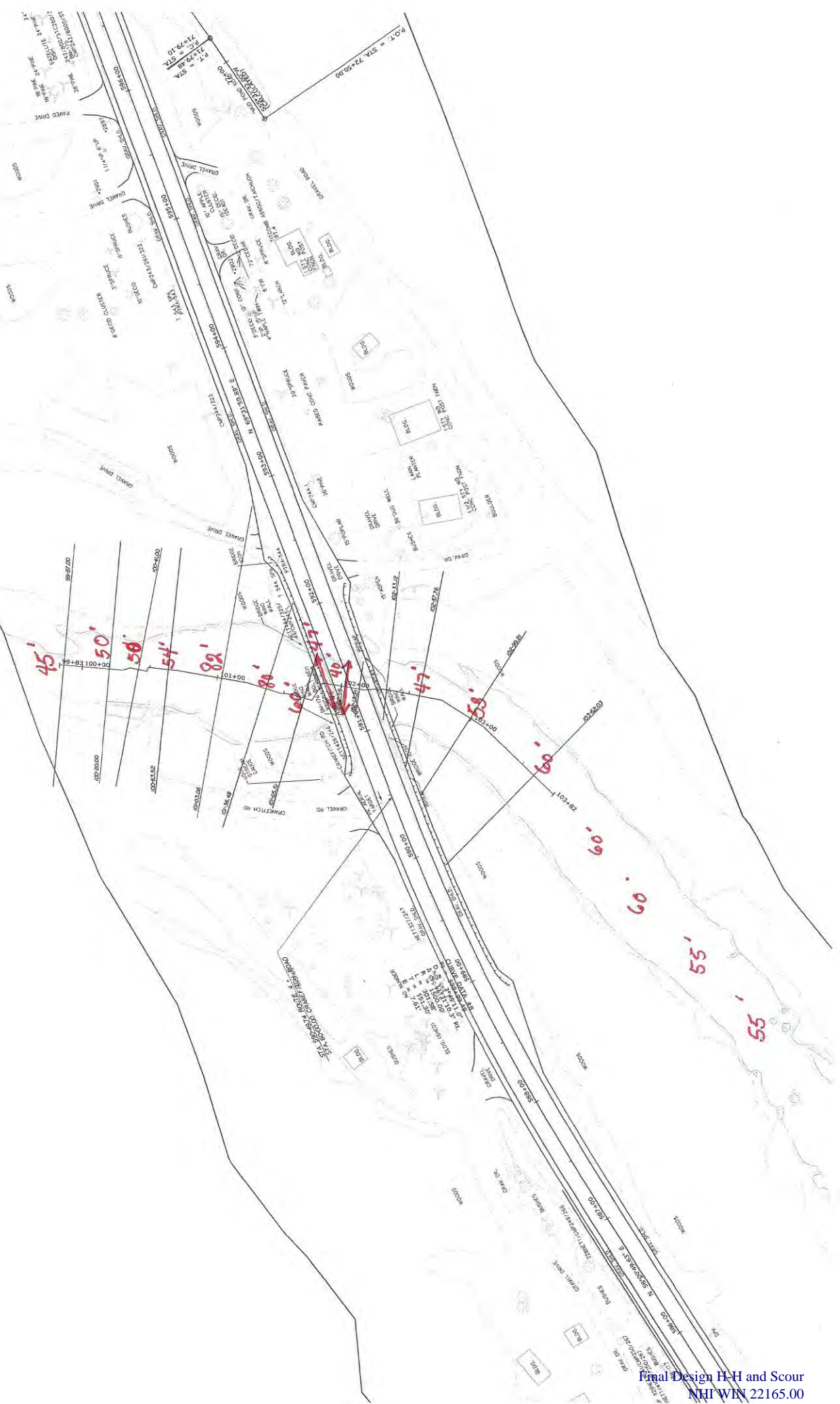
**1. Width of Channel at Time of Survey:** Data provided for rough check of channel width and is subject to water levels at the time the survey was taken. The data is not used in the average calculation used to derive bank full width as it can be subjective.

**2. Natural Geomorphic Channel:** Assessment of channel width based on important channel features such as slope, bedrock outcrops and the channel alignment without constrictions (bridge in this case). Data is considered in the average.

**3. Cross Section Survey Plot Data:** Width of channel is estimated from channel survey information. The bank edge of the channel is the change in slope in the topography; no specific water level is considered. Data is considered in the average.

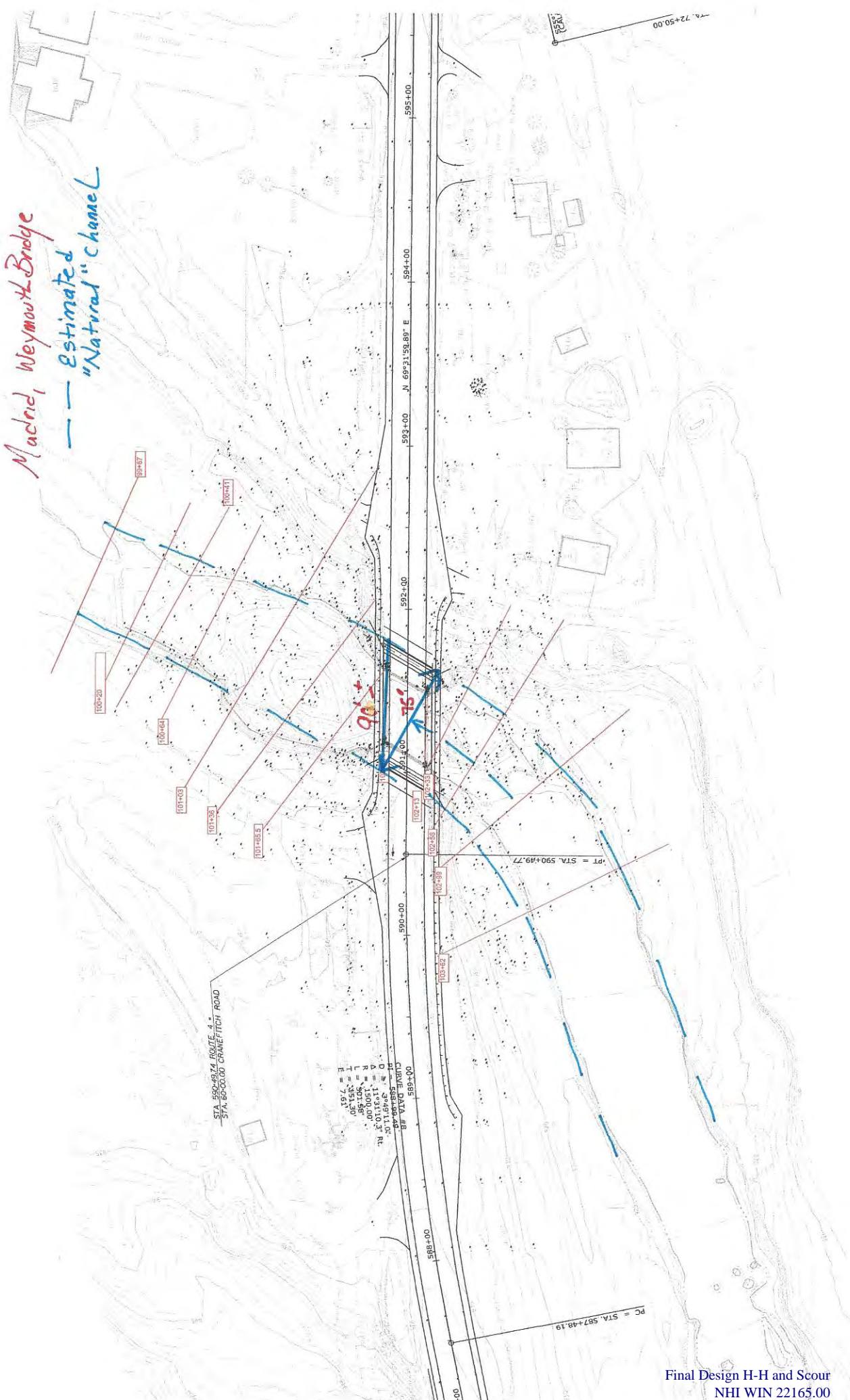
**4. Hydraulic Model HECRAS 1.1-year flow width:** Most reliable data for bank full width assessment. Using model HECRAS and the 1.1-year streamflow, channel width is calculated from the 1.1 year flow data output, intersection of water level and channel bank. Data is considered in the average.

Width of Stream at  
Time of survey

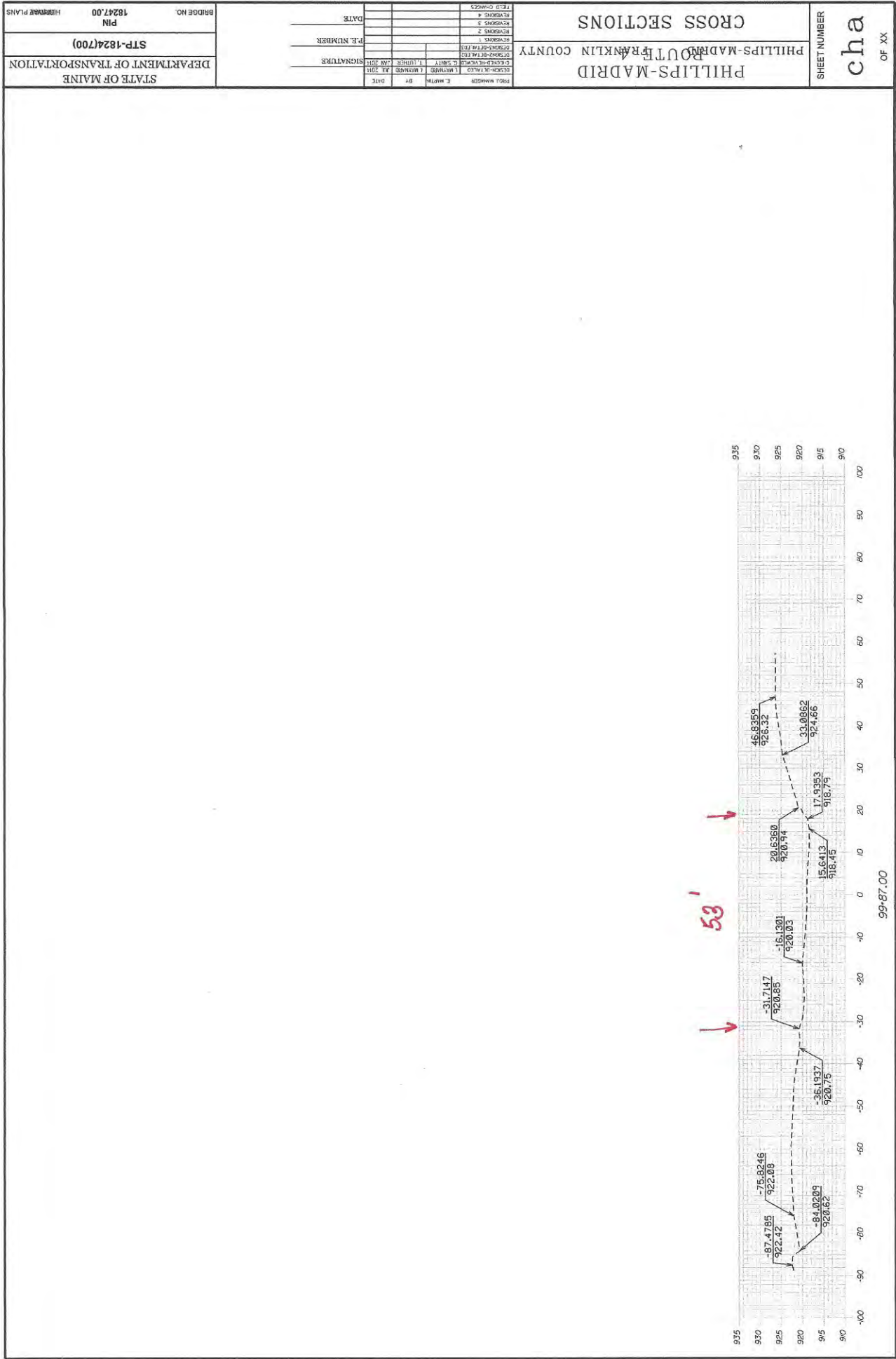


METHOD 2: Natural Channel Geomorphic Estimate

Madrid, Weymouth Bridge  
— Estimated  
"Natural" Channel

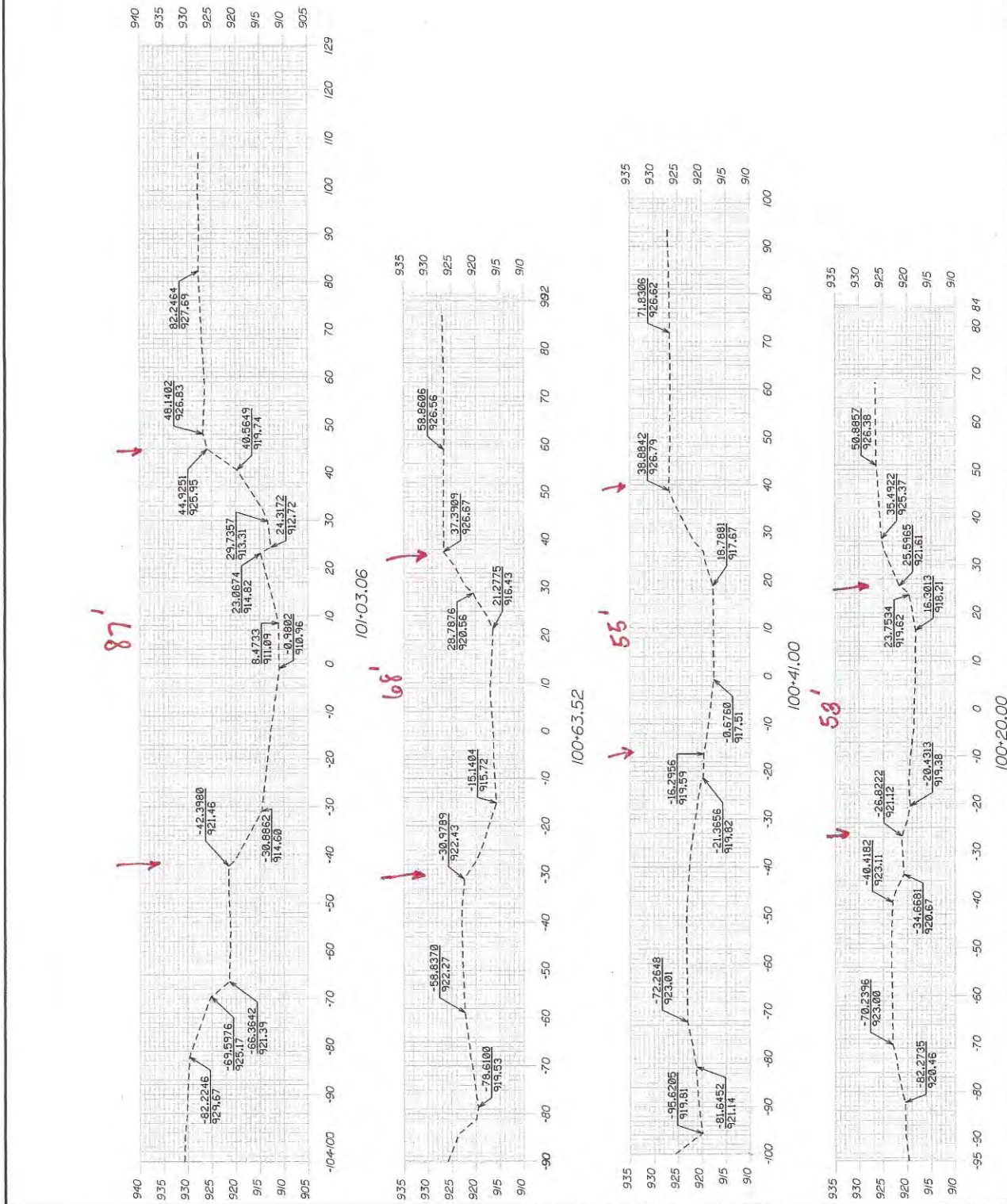


METHOD 3: Channel Cross Section Survey Plots



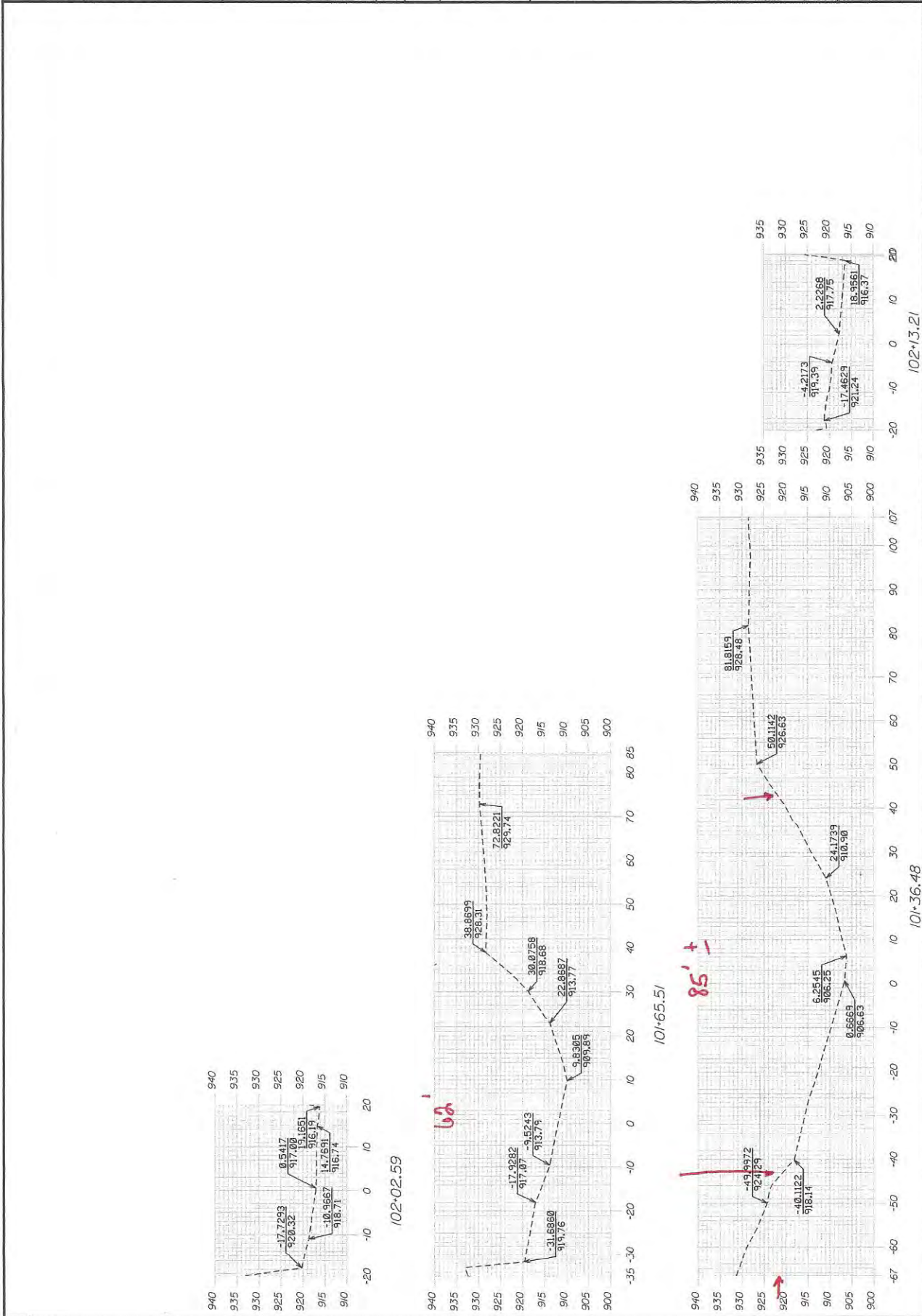
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PROJECT MANAGER C. MARTIN BY DATE		PROJECT MANAGER C. MARTIN BY DATE	
PROJECT NUMBER 1824(700)		PROJECT NUMBER 1824(700)	
STATE OF MAINE DEPARTMENT OF TRANSPORTATION		STATE OF MAINE DEPARTMENT OF TRANSPORTATION	
BRIDGE NO. 18247.00		BRIDGE NO. 18247.00	
PIN STP-1824(700)		PIN STP-1824(700)	

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 OF XX  
 SHEET NUMBER



Sta. 100+20.00 to Sta. 101+03.06

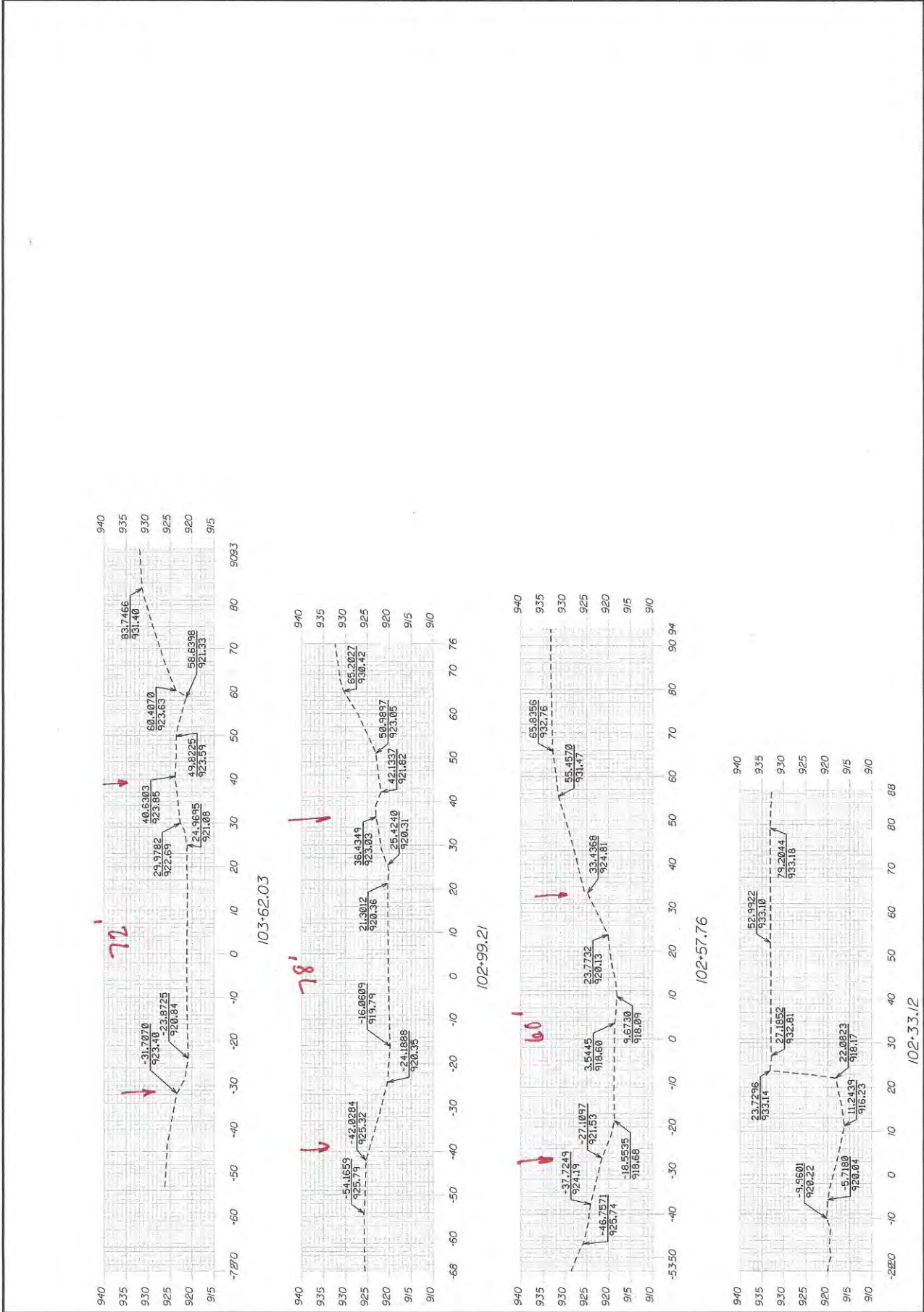
PHILLIPS-MADRID ROUTE 102 FRANKLIN COUNTY		CROSS SECTIONS PHILLIPS-MADRID ROUTE 102 FRANKLIN COUNTY	
PROJECT MANAGER: E. MARTIN DATE: 10/17/2014 CHECKED: J. WILSON DATE: 10/17/2014 DESIGNED: J. WILSON DATE: 10/17/2014 REVISIONS: 1 REVISIONS: 2 REVISIONS: 3 REVISIONS: 4		SHEET NUMBER <b>cha</b> OF XX	
STATE OF MAINE DEPARTMENT OF TRANSPORTATION STP-1824(700) PIN 18247.00		BRIDGE NO. 18247.00	



Sta. 101+36.48 to Sta. 102+13.21

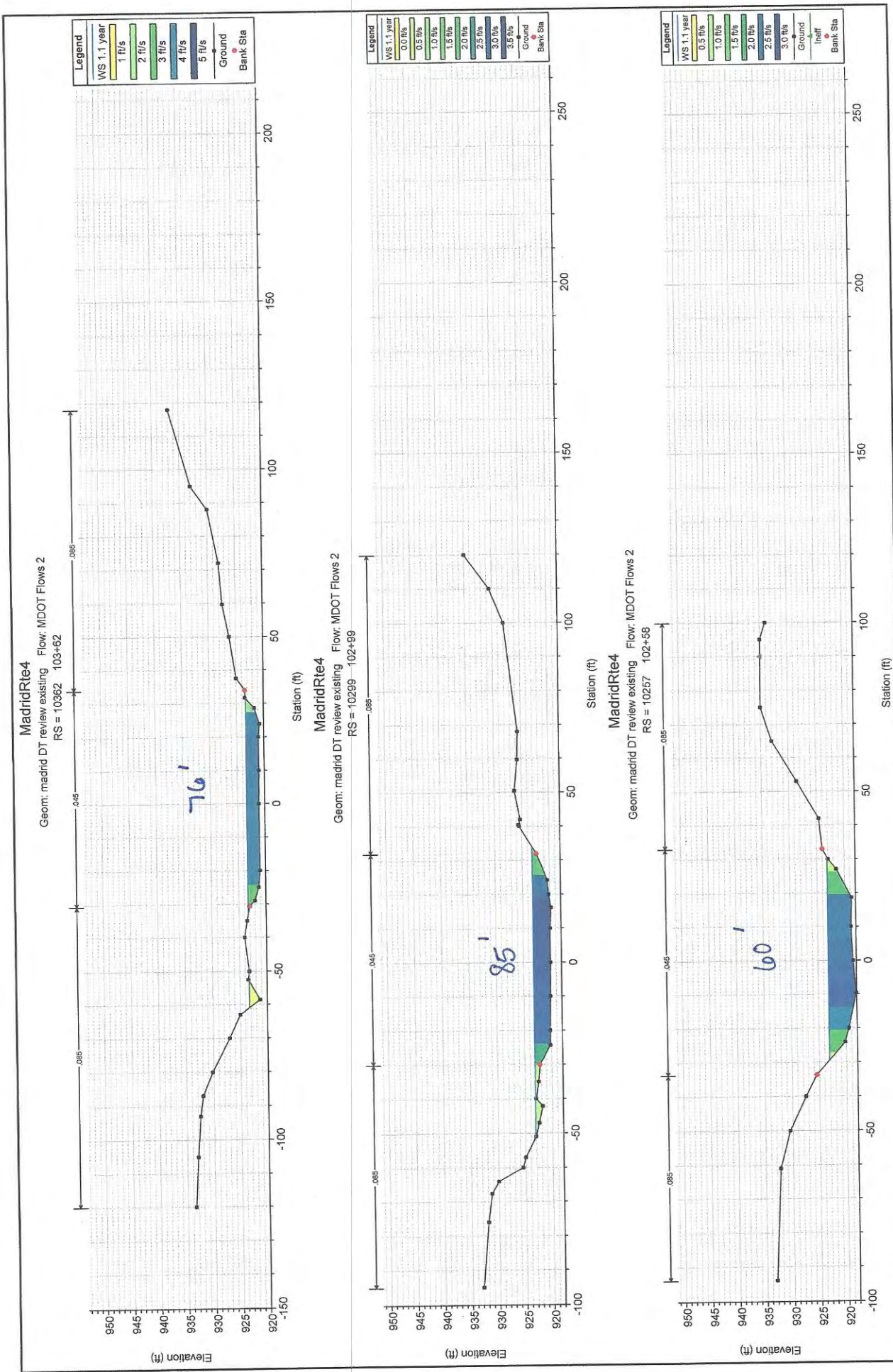
PHILLIPS-MADRID ROUTE 102 FRANKLIN COUNTY		CROSS SECTIONS PHILLIPS-MADRID ROUTE 102 FRANKLIN COUNTY	
PROJ. MANAGER E. MARTIN DATE 11/11/2014		PROJ. MANAGER E. MARTIN DATE 11/11/2014	
CHECKED BY J. MARTIN DATE 11/11/2014		CHECKED BY J. MARTIN DATE 11/11/2014	
DESIGNED BY J. MARTIN DATE 11/11/2014		DESIGNED BY J. MARTIN DATE 11/11/2014	
P.R. NUMBER 102-33.12		P.R. NUMBER 102-33.12	
DATE 11/11/2014		DATE 11/11/2014	
STATE OF MAINE DEPARTMENT OF TRANSPORTATION		STATE OF MAINE DEPARTMENT OF TRANSPORTATION	
BRIDGE NO. 18247.00		BRIDGE NO. 18247.00	
PIN STP-1824(700)		PIN STP-1824(700)	

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 SHEET NUMBER

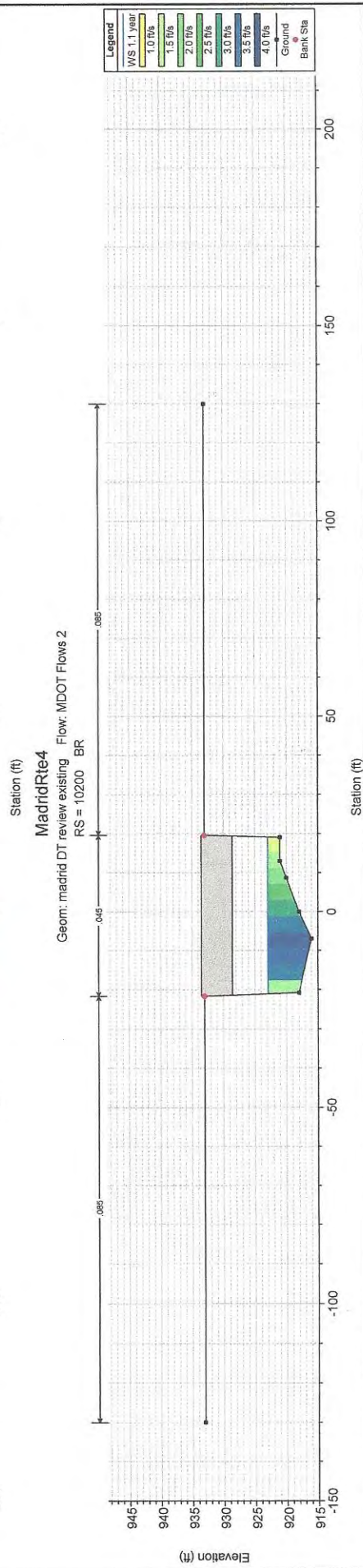
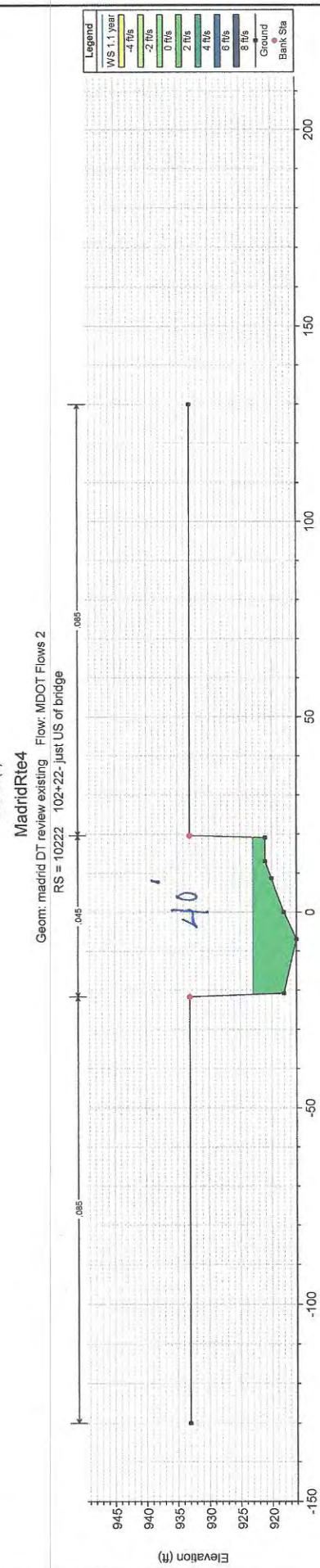
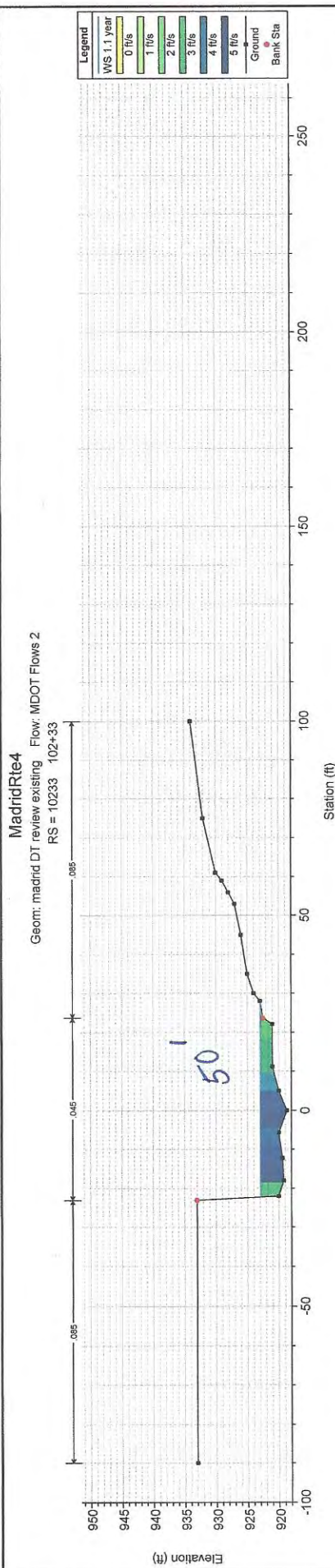


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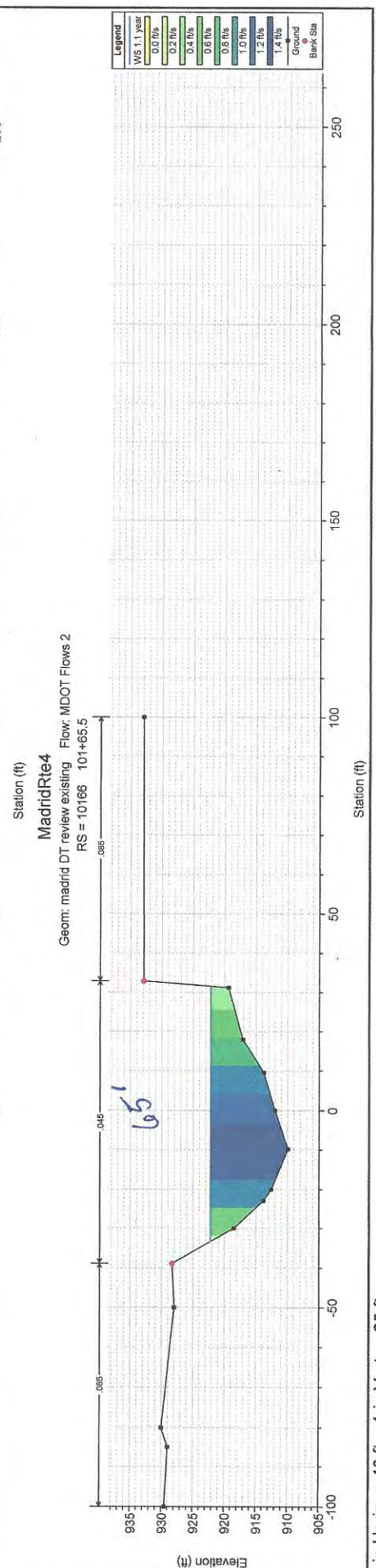
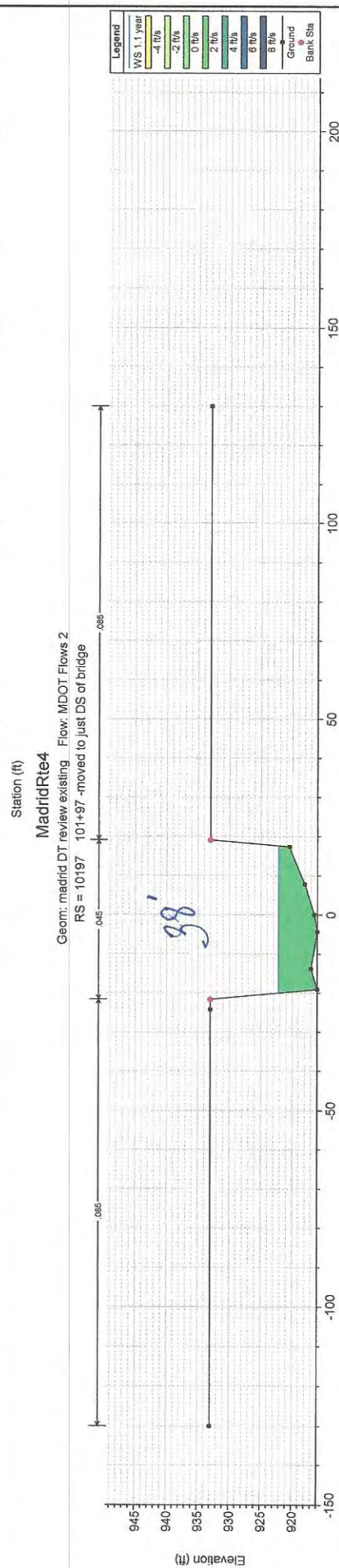
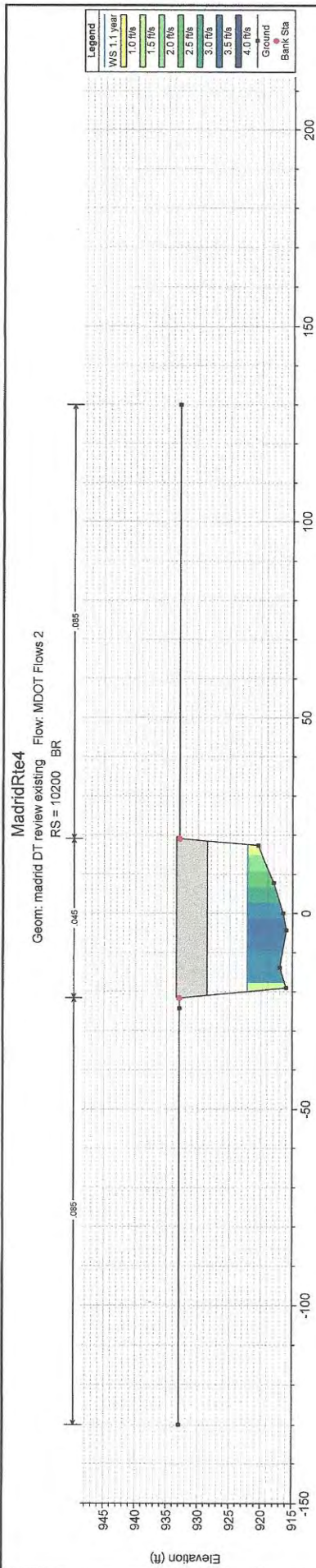
# METHOD 4: Hydraulic Model HECRAS 1.1-year flow width



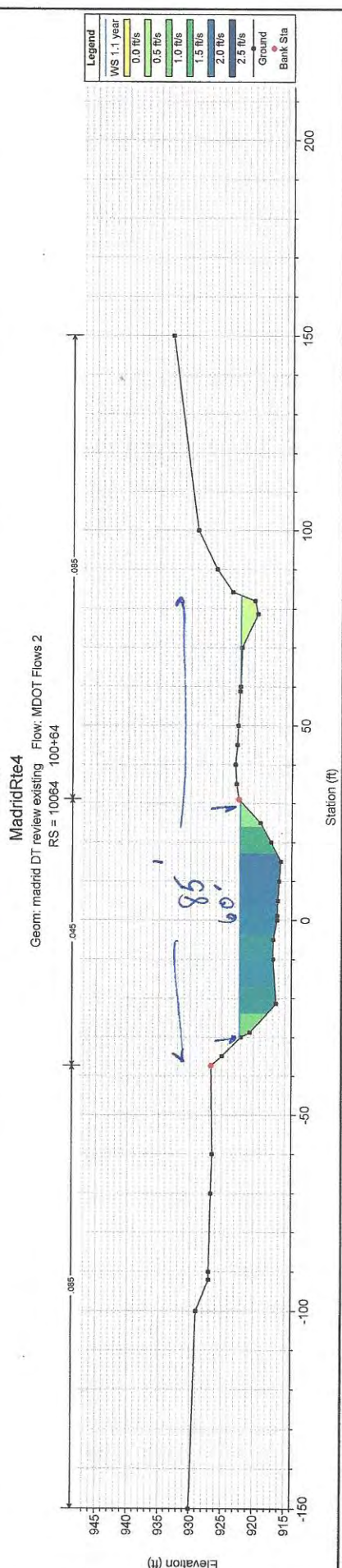
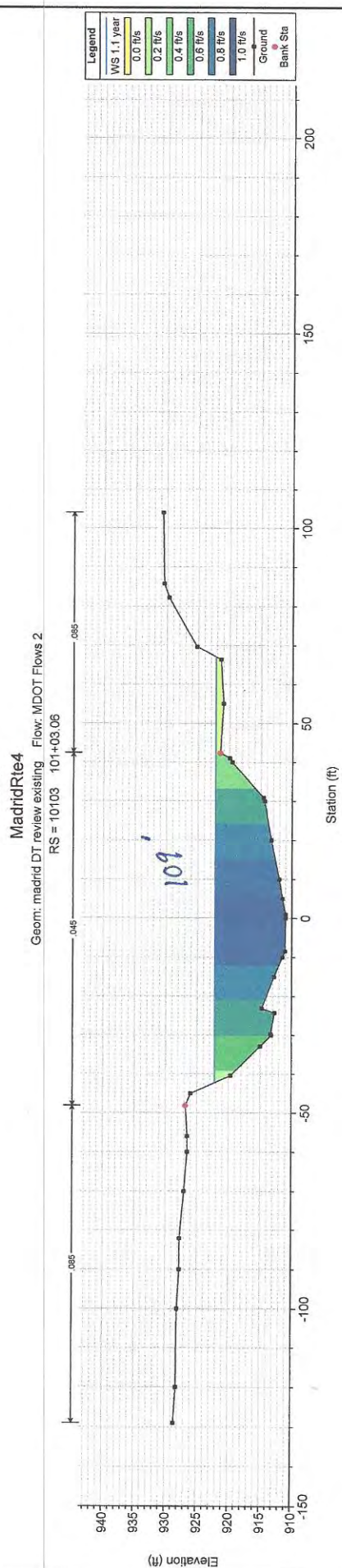
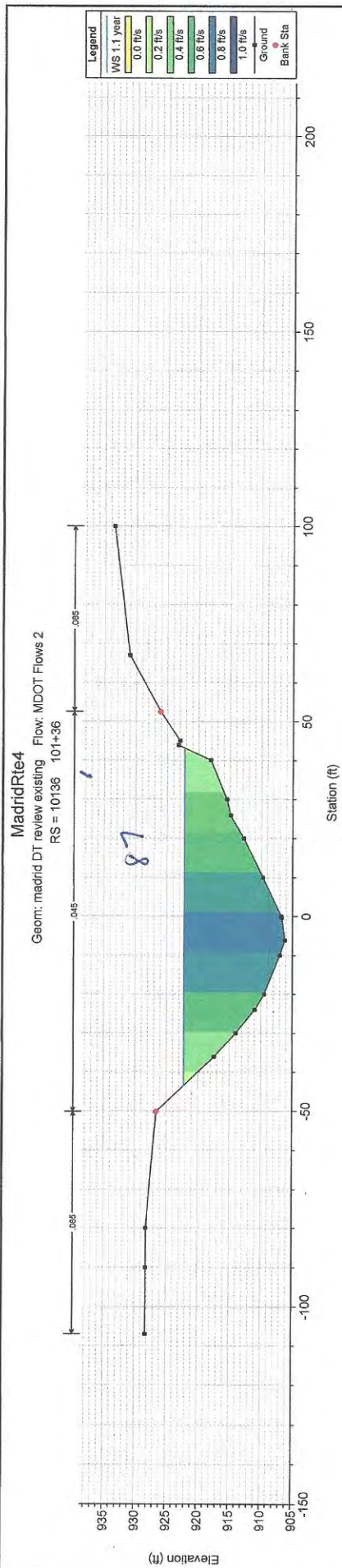
1 in Horiz. = 40 ft 1 in Vert. = 25 ft



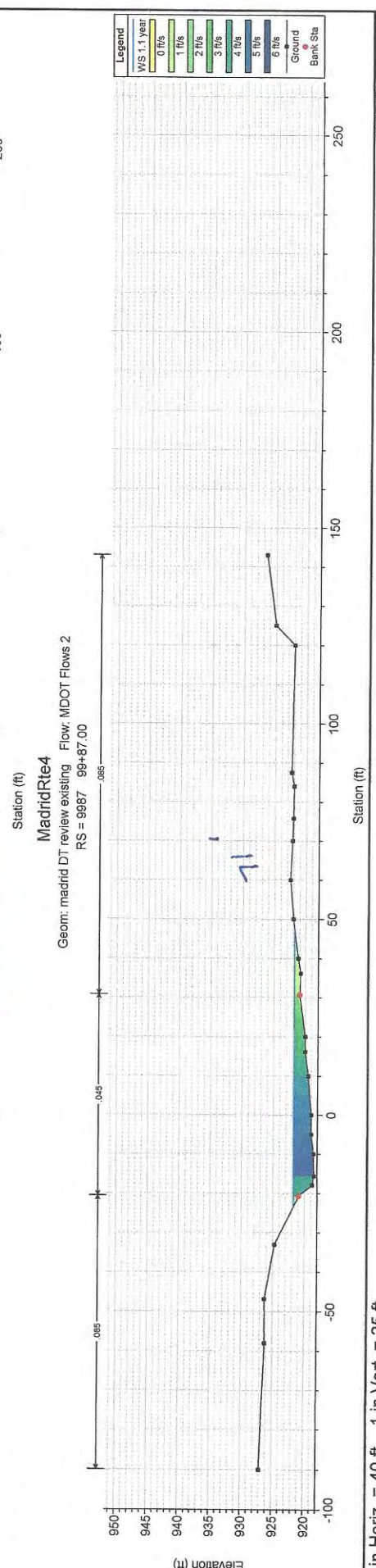
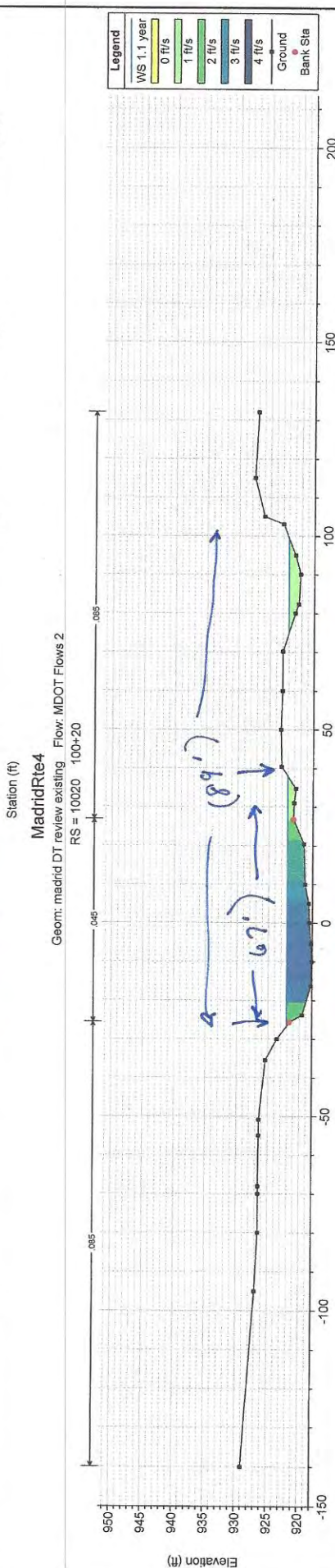
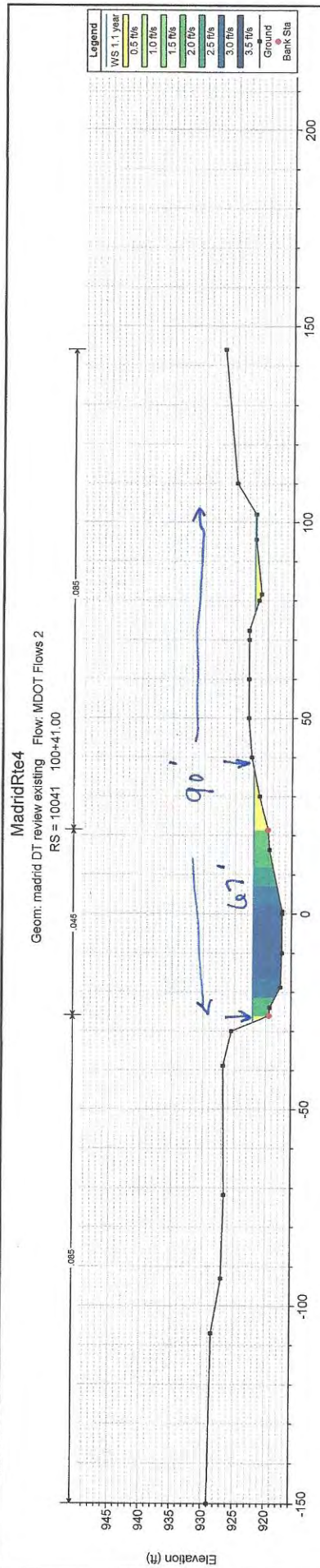
1 in Horiz. = 40 ft 1 in Vert. = 25 ft



1 in Horiz. = 40 ft 1 in Vert. = 25 ft

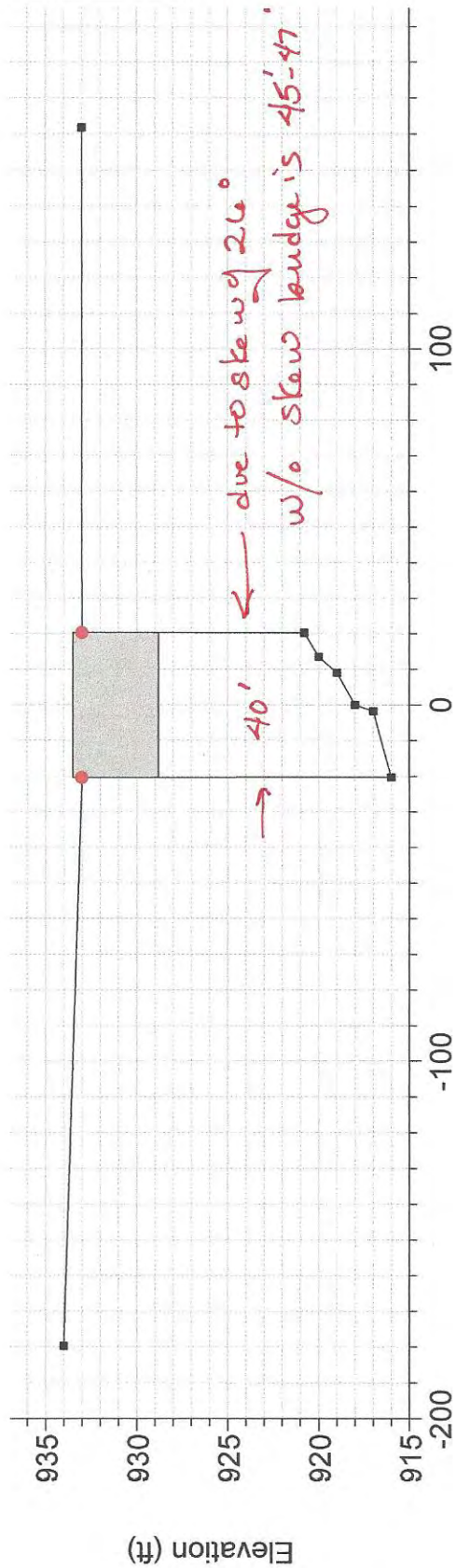


1 in Horiz. = 40 ft 1 in Vert. = 25 ft

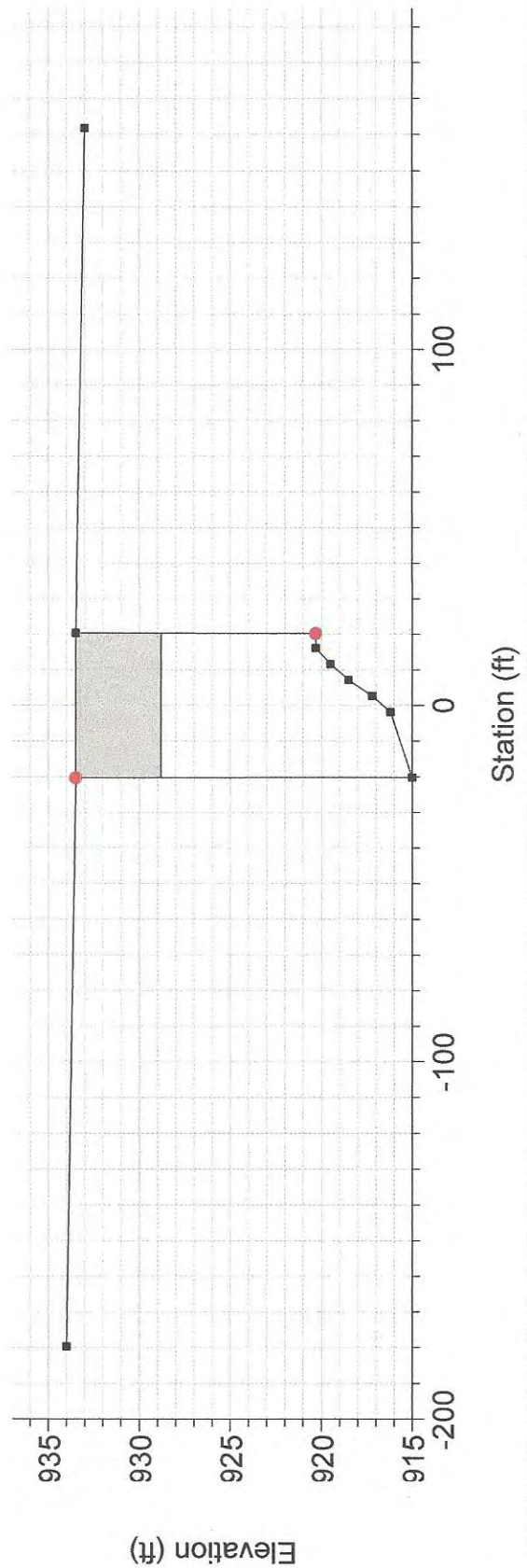


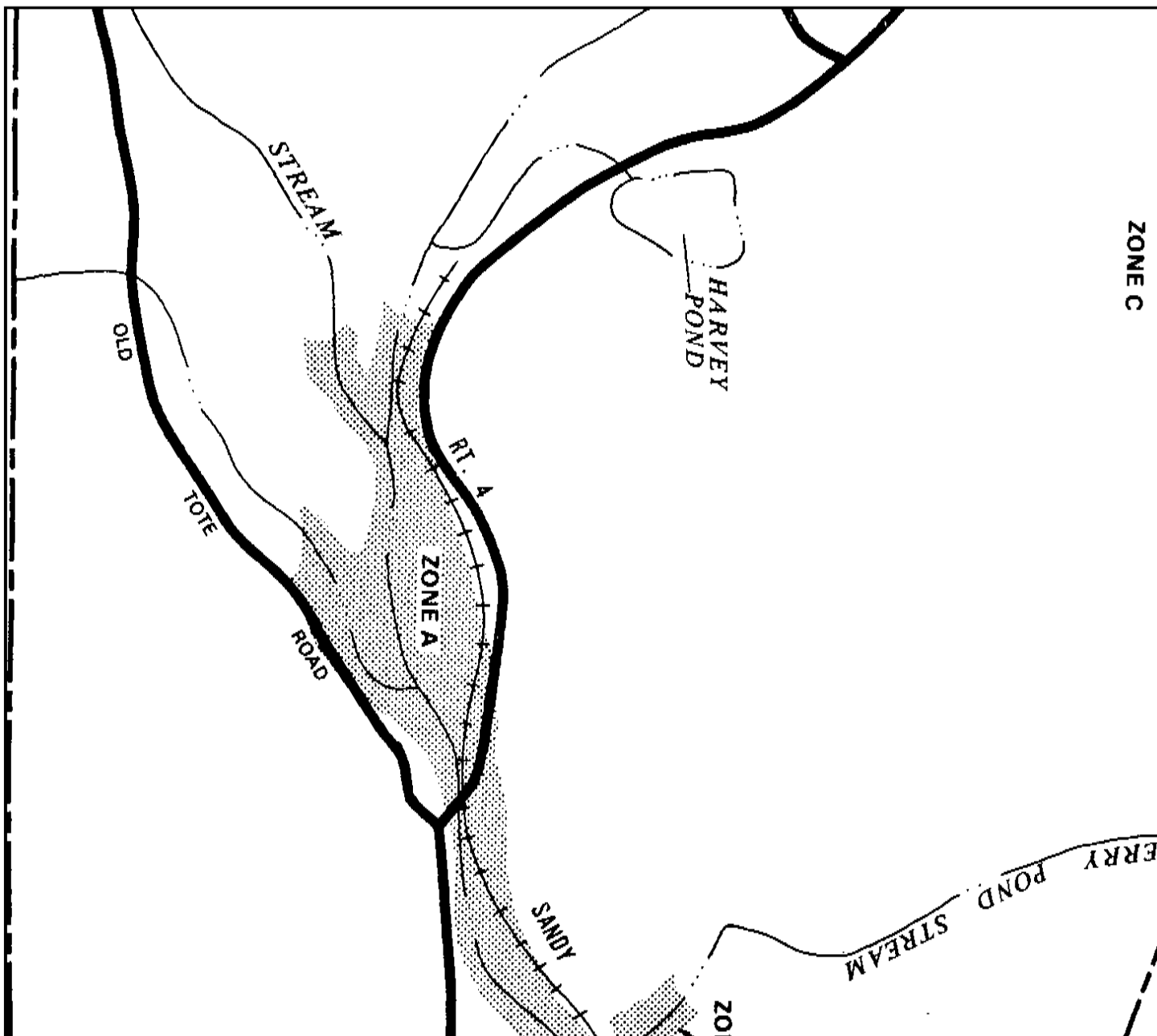
1 in Horiz. = 40 ft 1 in Vert. = 25 ft

RS=10200 Upstream (Bridge)



RS=10200 Downstream (Bridge)





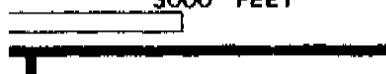
JOINS 09



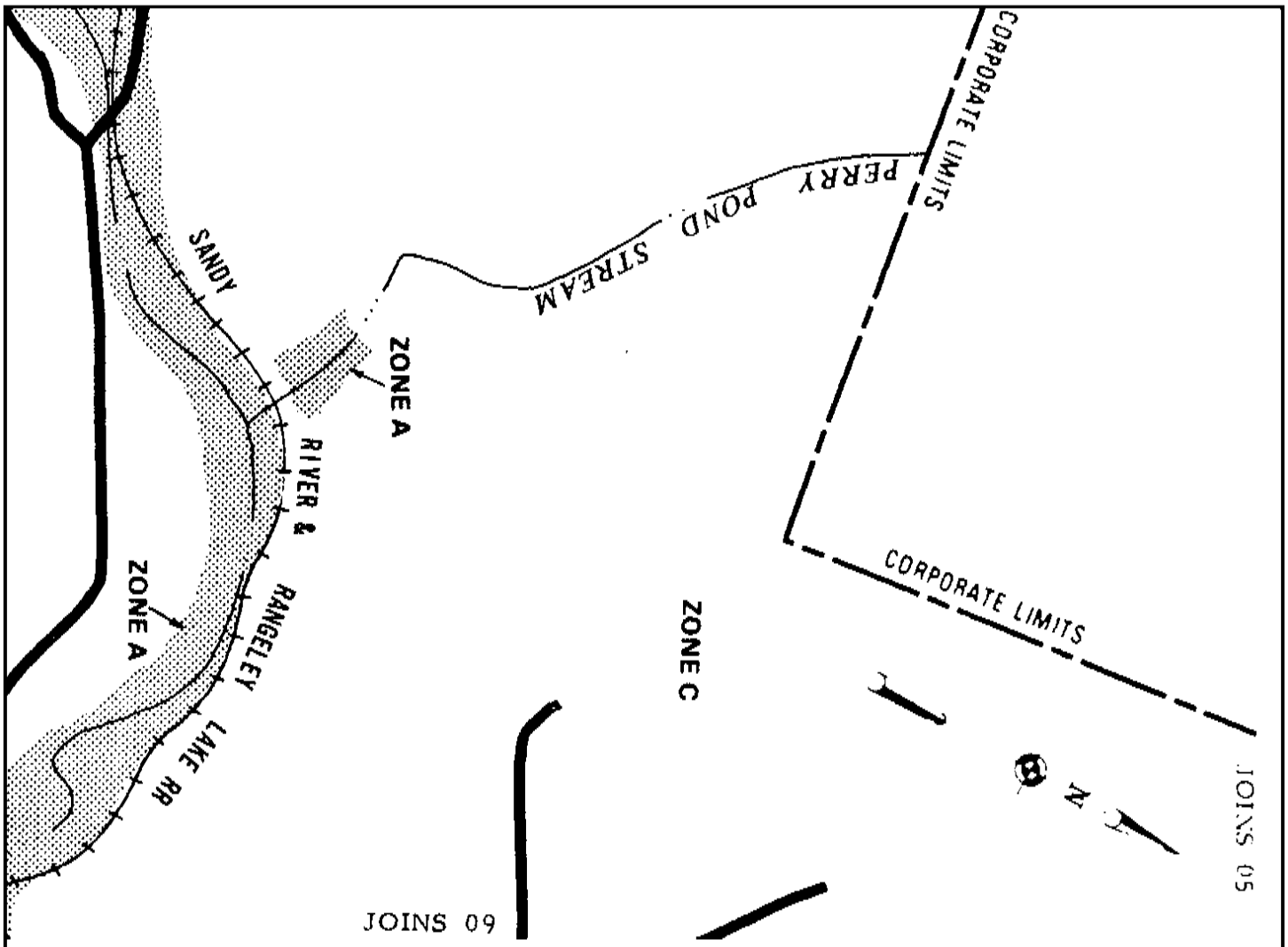
**federal emergency management agency**

**TOWN OF MADRID, ME**

3000 FEET

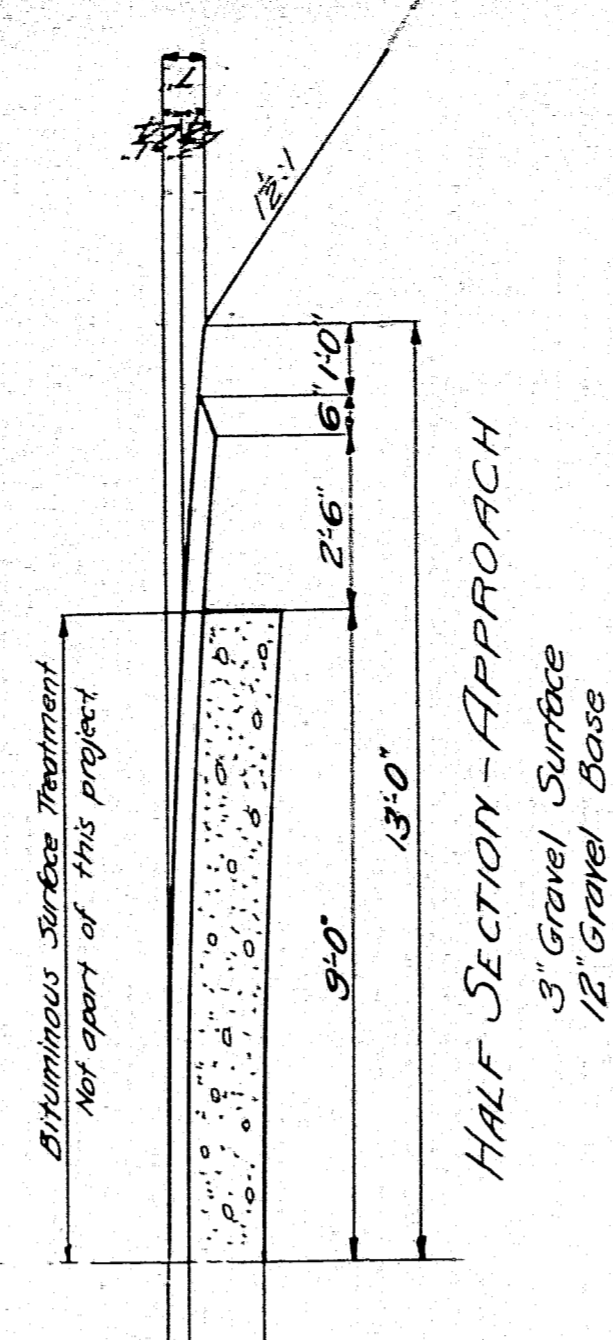
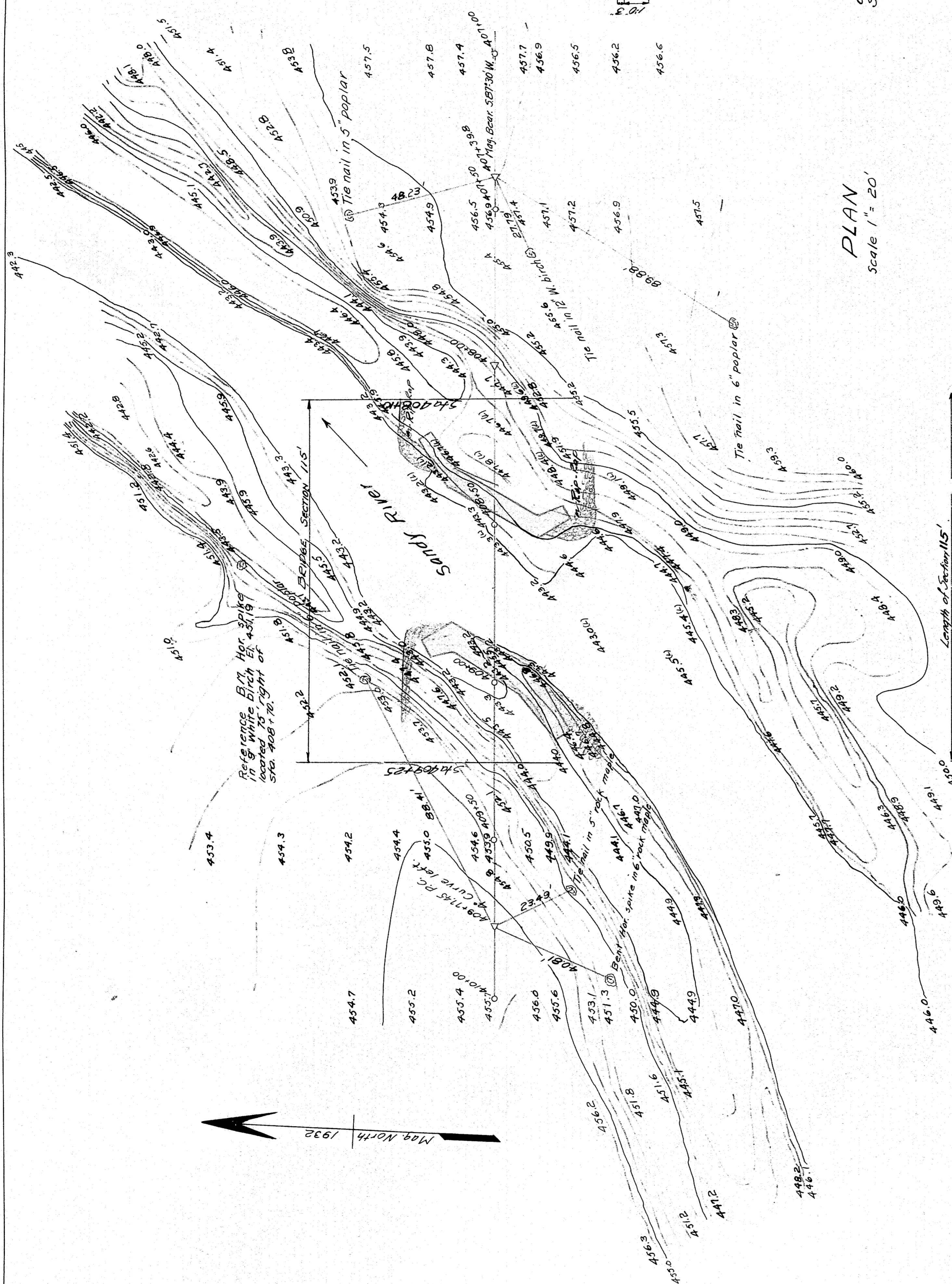


This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT Ch-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at [www.msc.fema.gov](http://www.msc.fema.gov)



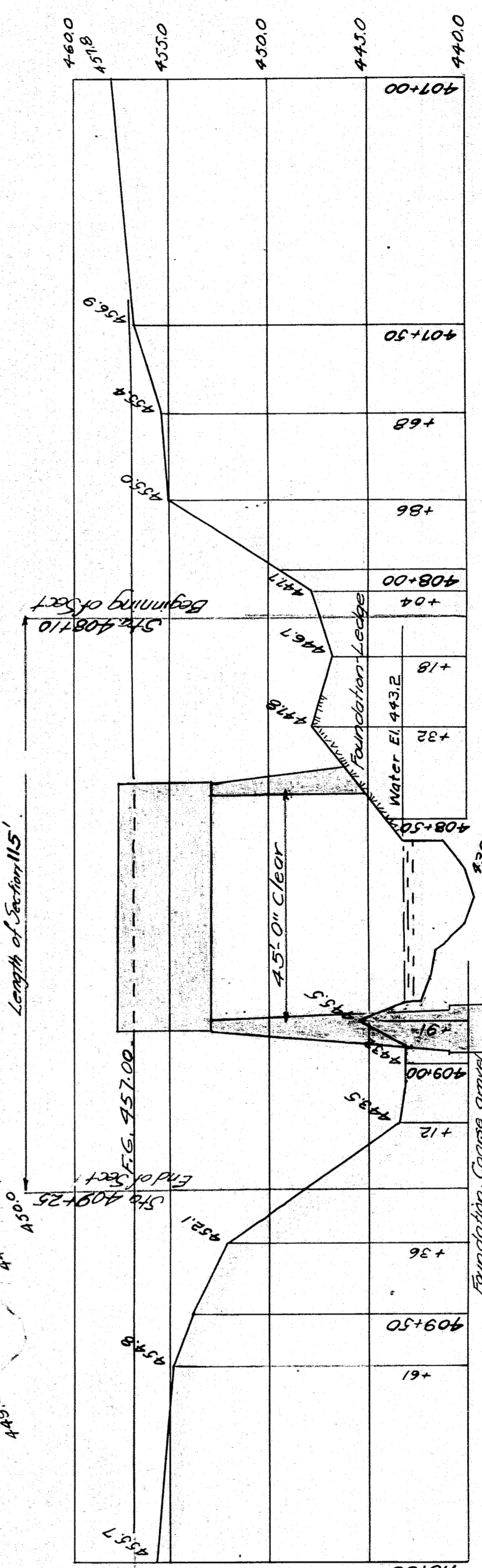
Management agency <b>MADRID, ME</b>	APPROXIMATE SCALE 	
	<b>FLOOD INSURANCE RATE MAP</b> <b>COMMUNITY NUMBER 230350 A</b>	EFFECTIVE DATE <b>AUGUST 19, 1985</b>

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT Ch-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at <a href="http://www.msc.fema.gov">www.msc.fema.gov</a>	JOINS 09		
	<b>federal emergency management agency</b> <b>TOWN OF MADRID, ME</b>		



Highway B.M. # 40  
Elev 460.63  
Point on N.E. side of ledge  
over  $\rightarrow$  mark 110' left of  
Sta. 406+40.

PLAN  
Scale 1" = 20'



PROFILE  
Scales Hor. 1"=20' Vert. 1"=5'

Town 04-16  
Bridge 2934

MAINE HIGHWAY COMMISSION  
BRIDGE DIVISION

WEYMOUTH BRIDGE  
OVER  
SANDY RIVER  
IN THE TOWN OF  
MADRID FRANKLIN CO.  
SURVEY

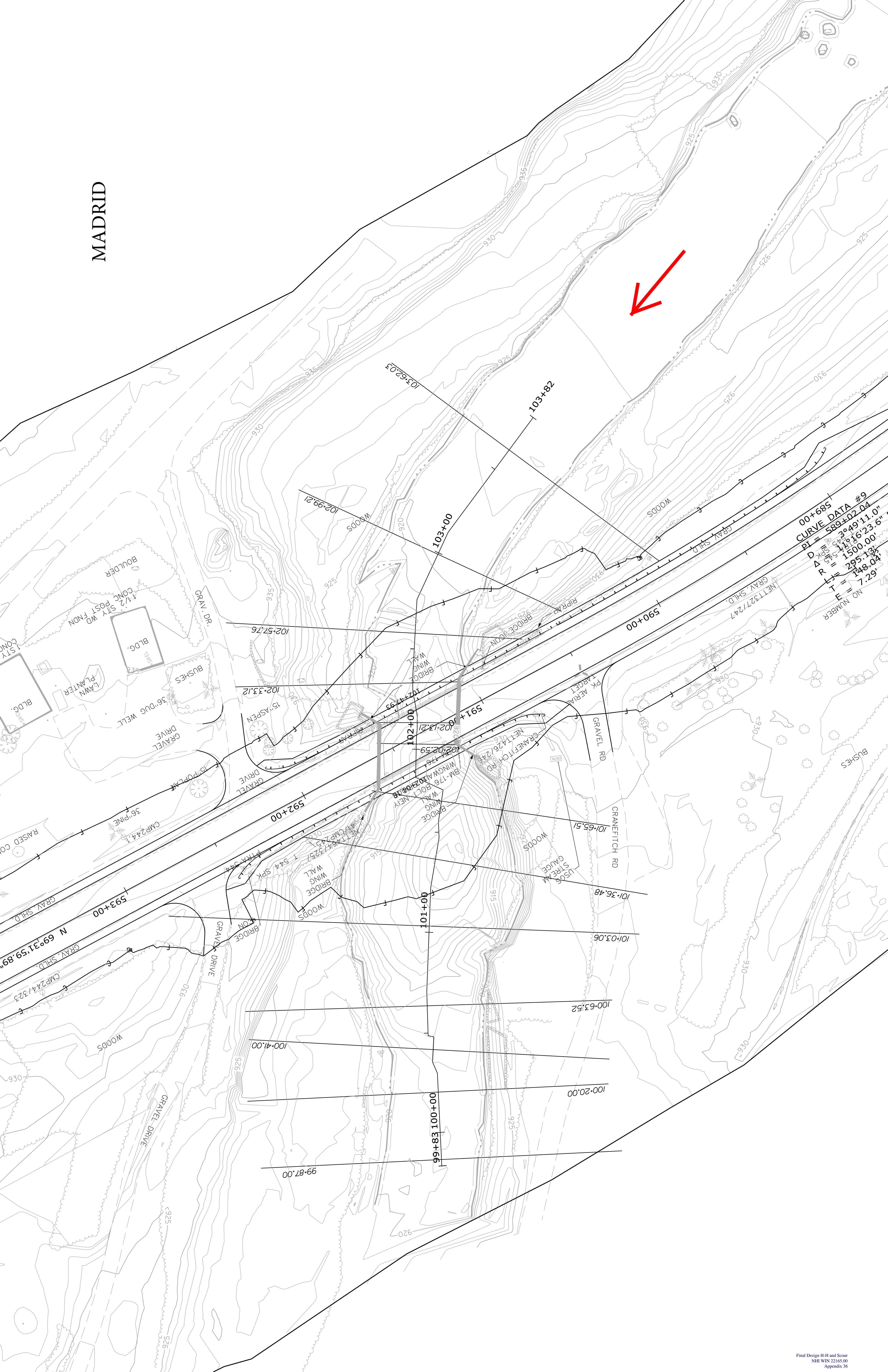
Sheet 2 of 7 Augusta Me. December 1932

17-127

Notes--  
Traffic Use existing bridge about  
500' upstream.  
Stream-- Swift current. Rapids  
and falls of 15' drop just  
upstream of center line. High  
water of Sept. 1932 Ft. 4519  
3' above planking of present  
bridge. High water of 1927  
reported at 3.5' above planking.  
For waterway study use former  
survey of existing bridge site.  
Foundation - Ledge.

Survey-Beckman  
Plotted-Whitten

MADRID



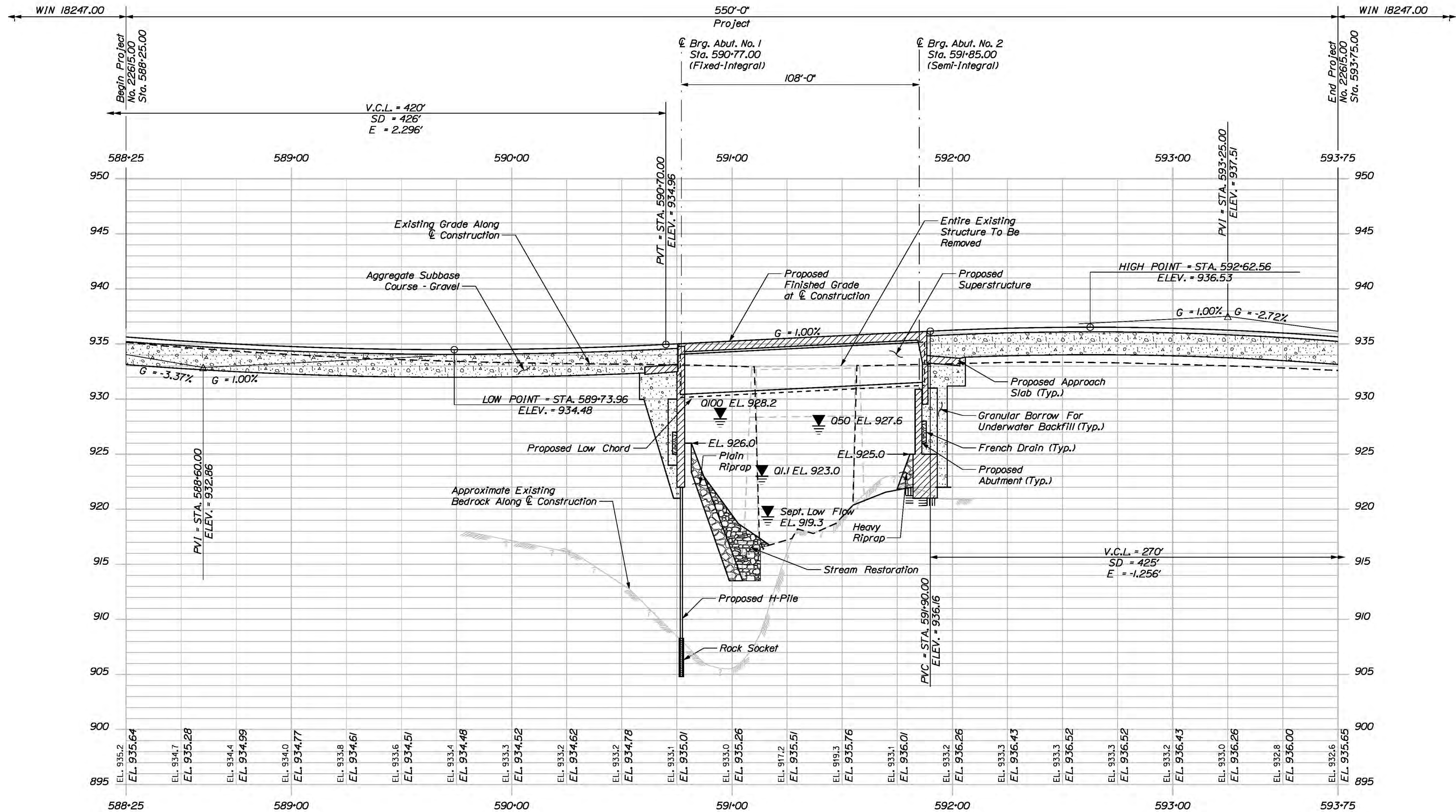
Final Design H-H and Scour  
NHI WIN 22165.00  
Appendix 36

Date: 2/6/2017

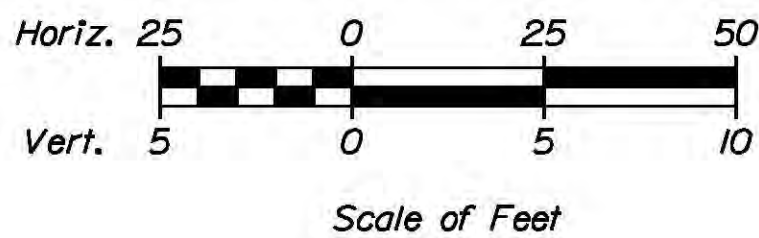
Username: dotaylor

Division: BRIDGE

Filename: ...\\00\\BRIDGE\\MSTA005\_Profile.dgn



PROFILE - ROUTE 4



PRELIMINARY  
NOT FOR CONSTRUCTION



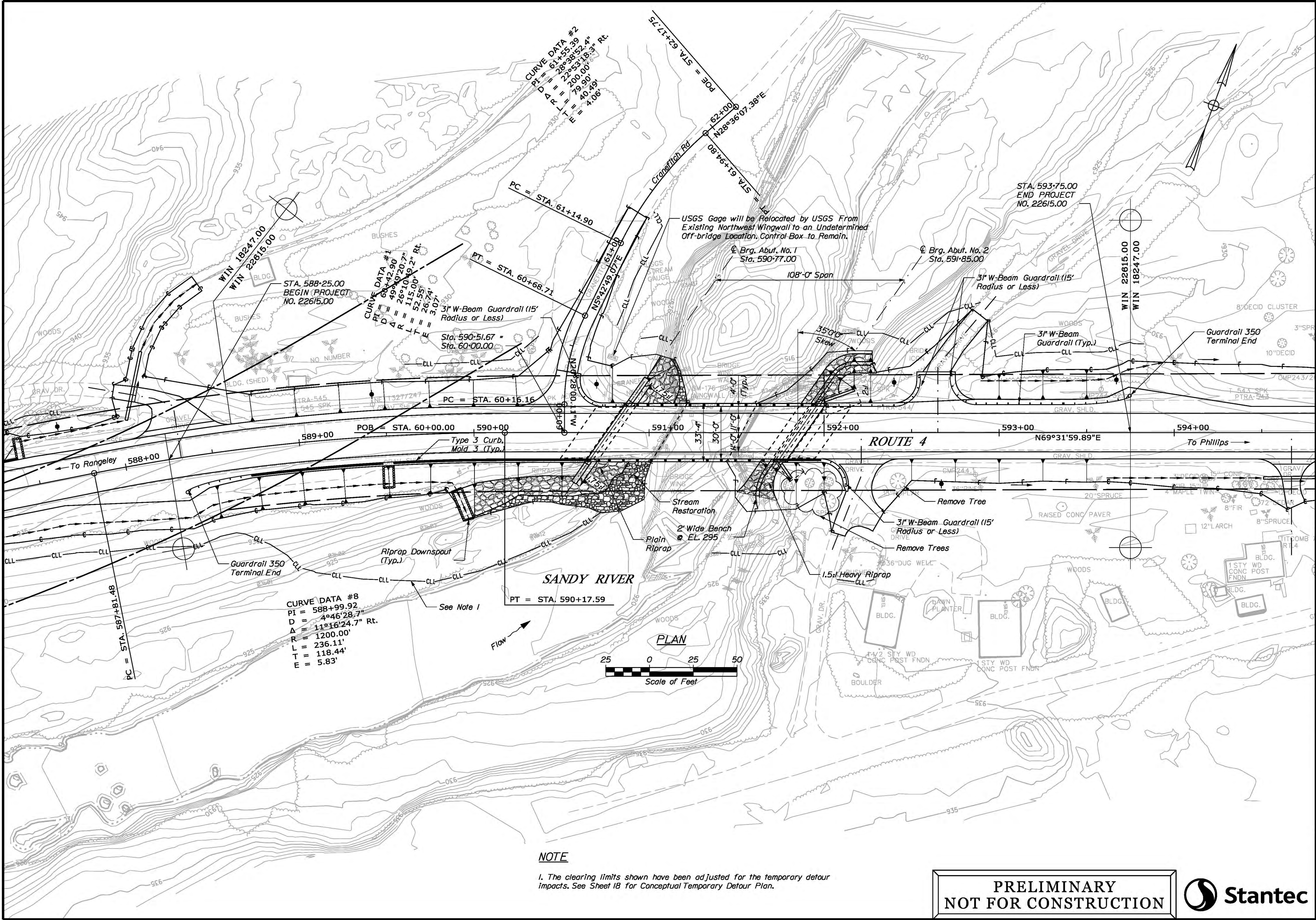
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Date:2/6/2017

Username: dotaylor

Division: BRIDGE

Filename: ...\\00\\BRIDGE\\MSTA\\004\_GenPlan.dgn



NOTE

1. The clearing limits shown have been adjusted for the temporary detour impacts. See Sheet 1B for Conceptual Temporary Detour Plan.

PRELIMINARY  
NOT FOR CONSTRUCTION



STATE OF MAINE DEPARTMENT OF TRANSPORTATION	STP-2261(500)		BRIDGE NO. 2934		WIN 22615.00		BRIDGE PLANS	
	DATE		SIGNATURE		P.E. NUMBER		DATE	
	BY		DATE		SIGNATURE		DATE	
WEYMOUTH BRIDGE SANDY RIVER FRANKLIN COUNTY MADRID	DESIGN-DETAILED	SUB	FEB 2017		DESIGNED	REVISED	DATE	
	CHECKED-REVIEWED	PIP	FEB 2017		DESIGNED	REVISED	DATE	
	DESIGNED	DET	FEB 2017		DESIGNED	REVISED	DATE	
	DESIGNED	DET	FEB 2017		DESIGNED	REVISED	DATE	
	DESIGNED	DET	FEB 2017		DESIGNED	REVISED	DATE	
SHEET NUMBER		4		OF 26				

# MAINE DEPARTMENT OF TRANSPORTATION

## BRIDGE SCOUR ASSESSMENT

### SUMMARY REPORT

Bridge Name: Weymouth	Town: Madrid	Bridge #: 2934
Feature Carried: Route 4	Stream: Sandy River	River Basin: Kennebec
Assessment By: D. Bryant	Assessment Date: 1/17/2010	
Check By: <i>D. Reynolds</i>	Check Date: 1/29/10	
MDOT PIN: 15631.10	TYLI Project No: 411588.10	

#### NOTES

- Observations left and right are facing downstream -
- Elevations refer to bridge plan datum unless otherwise noted -
- The information shown in this report is obtained from available MDOT bridge plans and records, supplemented by field review -

#### 1. SUMMARY AND CONCLUSIONS

The following factors have the most significant influence on the recommendations:

##### Positive Influence:

- Right abutment/streambed on ledge.
- Dam control 3 miles upstream.
- Bridge circa 1933 has probably withstood at least one >100 year flood event in 1987 and two > 50 year storms with no major recorded negative impacts in inspection reports.

##### Negative Influence:

- Poor flow alignment.
- Left abutment on spread footing and in channel. Minor abutment concrete cracks and road settlement.
- No available historic flood data.
- Compared to construction plans, up to 4' of scour at left abutment, thalweg shifted to left and is now near abutment. Approximately 1.5' exposure left abutment, downstream.
- MaineDOT inspection reports indicate ongoing scour issues with left abutment, high velocities.
- Scourable bed material. Some bank erosion and scour hole observed downstream.
- Thalweg at bottom of left abutment footings.
- Bridge opening slightly narrower than channel or floodplain width. Steep upstream banks.
- Debris noted in area - mostly downstream.
- Left abutment riprap in fair condition, inspection reports note some missing.
- High water on plans very close to low chord elevation.

##### Conclusions:

- Difficult access to abutments due to steep banks. Headroom adequate but water along left abutment is deep. Temporary stream diversion likely required during low flow.
- Grout mat or concrete cable mat likely effective countermeasure alternatives to riprap due to deep water.
- Hydraulic and scour evaluation recommended. Three pass/fail abbreviated POA criteria were not met. Full POA recommended. Installation of countermeasures required to improve Item 113 rating.

#### 2. RECOMMENDATIONS

##### a. Scour Vulnerability (Long term, Contraction, Abutment, Pier):

- Moderate potential for contraction scour. Low potential for right abutment scour, high potential for left abutment scour. Moderate potential for pressure flow. Footing exposure up to 1.5 feet, thalweg at bottom of left, spread footing. Poor flow alignment. Scouring noted in inspection reports. Scourable bed. Riprap in fair condition, but some is missing. Vertical wall abutments with exposed left footing in channel.

##### b. Recommended NBI Ratings:

Item 60: 3      Item 61: 3      Item 71: 8      Item 113: 3

c. POA Recommended (X):      Full X      Abbreviated \_\_\_\_\_

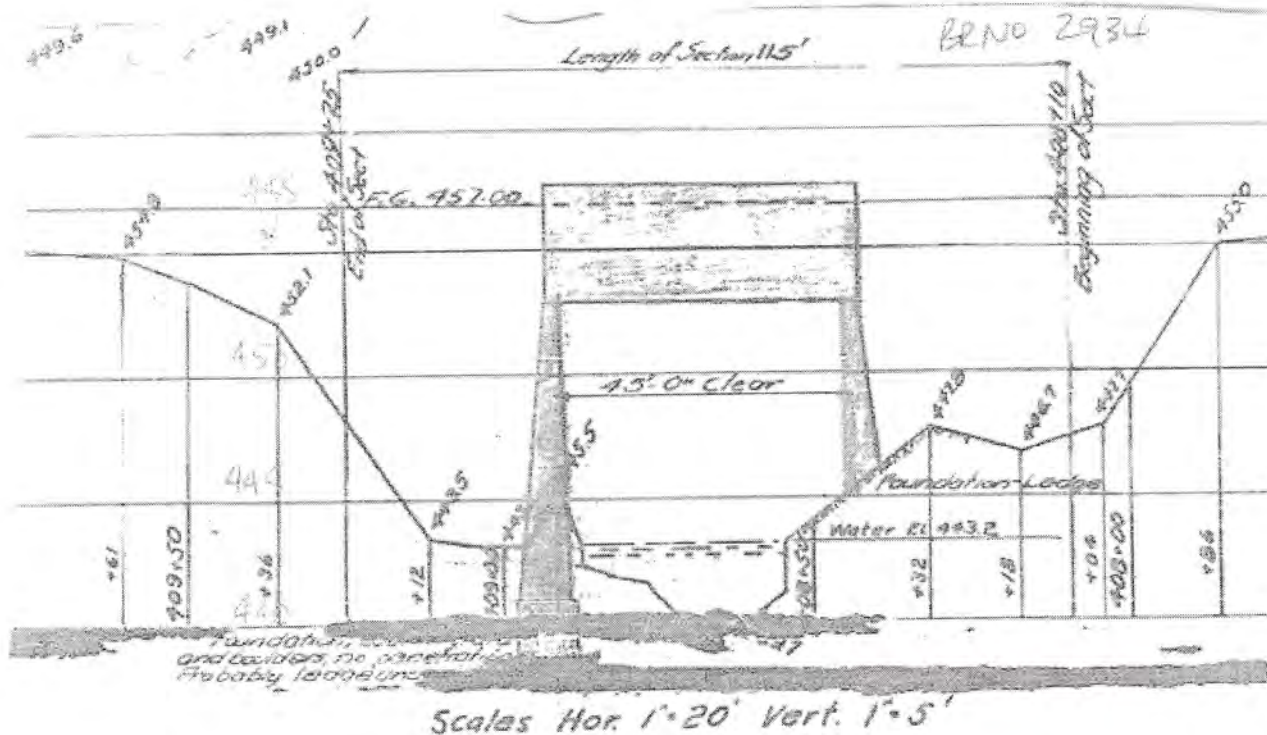
# Scour Evaluation Report

Division	TOWN	BRNAME	BRNO	Field Review/ Date:
7	Madrid	Weymouth	2934	12/2/88
ITEM113	FNDAT	Stream	PLANLOC	Year Built
3	32N	Sandy River		1933

## FIELD/OFFICE REVIEW NOTES

Riprap 1936 53, 57. Channel repairs 1968. Stone blanket 1983. Plan Notes: Foundation ledge. Stream rapid, falls. Flow is moving slowly North. Rapid flow. Increased channel and debris within below mainrap (35 ft. above bridge). RATE FROM 112 to 3 REVERSE LOWEST FINEST PLAN. PLANS AND/OR PHOTOGRAPHY FROM 15 TO 20 FEET FROM BRIDGE. APPEARS THAT CHANNEL WAS DEEPENED 2' IN AREA. THERE IS A LARGE SCOUR HOLE IN

## PLANS



# Contraction Scour

filled in variables

key computed variables

$$V_c = K_u * y^{(1/6)} * D^{(1/3)}$$

Critical Velocity in fps

Ku	6.19
D50 mm	1
D50 M	0.001
y	5.6
	8.6

S	0.002
	0.003

ave depth in us section

y	y ^.167	D	D^.333	Ku	VC
5.6	1.33336	0.001	0.100231	6.19	0.827252
8.6	1.43239	0.001	0.100231	6.19	0.888693

	V*/T<.5	V*/T>.5<2	V*/T>2
k1	0.59	0.64	0.69
V*=(gyS1)^.5	100	0.6005331	
	500	0.91146037	
T, fig 6.8	mps	0.12	
	fps	0.0375	
V*/T	100	16.0142159	
	500	24.3056098	

## Live Bed Scour

y1	ave depth in US channel
y2	ave depth in contracted section after scour
y0	existing depth in contracted section
Q1	flow in US channel section
Q2	flow in contracted channel section
W1	bottom width of US main channel transporting bed material
W2	bottom width of main channel in contracted section
Ys	scour depth

$$y2/y1 = (Q2/Q1)^{(6/7)} * (W1/W2)^{k1}$$

	100	500
	5.6	8.6
	6.1	9.7
	7.4	8.4
	3790.0	4950.0
	4124.0	5604.0
	85.0	85.0
	83.0	83.0
	0.5	1.1

$$(Q2/Q1)^{(6/7)}$$

$$(W1/W2)^{k1}$$

	100	500	100	500
	1.075077	1.112228	1.016565	1.016565